









Service Life Design of Bridges

Workshop W05 – International Bridge Conference Mike Bartholomew, P.E.

Technology Director, North American Bridges

CH2M

June 7, 2016





Presentation Overview

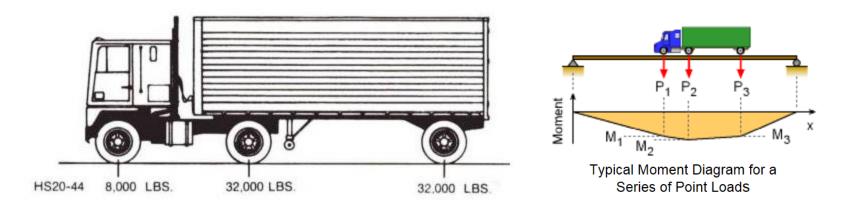
Service Life Design – What is it?

Historical Background – What's been done?

- Current Status / Gaps What's being done?
- Proposed Research on Service Life Design What's next?

Service Life Background

- Bridge Design focuses on structural engineering
 - Determining loads, sizing components, and selecting materials by their strength properties (f'c, fy, etc.)



 Extremely important, but does little to ensure that a structure will remain in use for a given period of time

Service Life Background

- When a structure reaches the end of its life
 - The cause is primarily from material deterioration





Due to the environmental exposure conditions

Service Life Design Principles

All materials deteriorate with time

Every material deteriorates at a unique rate

- Deterioration rate is dependent on:
 - Environmental exposure conditions
 - Material's protective systems durability properties

Service Life Design (SLD)

- Design approach to resist deterioration caused by environmental actions
 - Also called Durability Design
 - Often referred to as Design for 100-year Service Life
- Not designing for the Service Limit States I, II, and III per LRFD 3.4

Service Life Design (SLD)

- Similar to strength design to resist structural failure caused by external loads
 - External Loads ← → Environmental Actions

- Material Strength ← → Durability Properties
- Both strength and Service Life Designs satisfy scientifically based modeling equations

Goals of Service Life Design

- Owners Need assurance that a long-lasting structure will be designed, built, and operated (Effective use of public funding \$\$)
- Engineers/Contractors/Asset Managers Need quantifiable scientific methods to evaluate estimated length of service for bridge components and materials

Service Life Background

 Significant research has been completed over the past 25 years on how materials deteriorate with time (particularly reinforced concrete).

 Mathematical solutions have been developed to model deterioration behaviour.

Past Practice – 1996-2000

ACI 365.1R-00

Service-Life Prediction—State-of-the-Art Report

Reported by ACI Committee 365

| James R. Clifton*† Chairman | | Dan J. Naus ^{*‡} Secretary | |
|--------------------------------|---------------|--|--|
| S. L. Amey* | M. Geiker | D. G. Manning | |
| J. P. Archibald | C. J. Hookham | P. K. Mukherjee | |
| N. R. Buenfeld | W. J. Irwin | J. Pommersheim | |
| P. D. Cady" | A. Kehnemui | M. D. Thomas | |
| C. W. Dolan | | R. E. Weyers* | |

This report presents current information on the service-life prediction of new and existing concrete structures. This information is important to both the owner and the design professional. Important factors controlling the service life of concrete and methodologies for evaluating the condition of the existing concrete structures, including definitions of key physical prop-erties, are also presented. Techniques for predicting the service life of concrete and the relationship between economics and the service life of structures are discussed. The examples provided discuss which service-life techniques are applied to concrete structures or structural components. Finally, needed developments are identified.

Keywords: construction; corrosion; design; durability; rehabilitation; repair; service life

CONTENTS

Chapter 1-Introduction, p. 365.1R-2

- 1.1—Background
- 1.2-Scope
- 1.3-Document use

ACI Committee Reports, Guides, Standard Practices, and Commentaries are intended for guidance in planning, designing, executing, and inspecting construction. This document is intended for the use of individuals who are competent to evaluate the significance and limitations of its content and recommendations and who will accept responsibility for the application of the material it contains. The American Concrete Institute disclaims any and all responsibility for the stated principles. The Institute shall not be liable for any loss or damage arising therefrom. Reference to this document shall not be made in contract documents. If items found in this document are desired by the Architect/Engineer to be a part of the contract documents, they shall be restated in mandatory language for incorporation by the Architect/Engineer.

Chapter 2—Environment, design, and construction considerations, p. 365.1R-3

- 2.1—Introduction
- 2.2-Environmental considerations
- 2.3—Design and structural loading considerations
- 2.4—Interaction of structural load and environmental effects
- 2.5-Construction-related considerations

Chapter 3-In-service inspection, condition assessment, and remaining service life, p. 365.1R-11

- 3.1—Introduction
- 3.2-Evaluation of reinforced concrete aging or degrada-
- 3.3-Condition, structural, and service-life assessments
- 3.4-Inspection and maintenance

Chapter 4—Methods for predicting the service life of concrete, p. 365.1R-17

- 4.1—Introduction
- 4.2-Approaches for predicting service life of new concrete
- 4.3-Prediction of remaining service life
- 4.4-Predictions based on extrapolations
- 4.5—Summary

Chapter 5-Economic considerations, p. 365.1R-24

- 5.1—Introduction
- 5.2-Economic analysis methods
- 5.3-Economic issues involving service life of concrete

ACI 365.18-00 became effective January 10, 2000. Copyrigh D 2000, American Concrete Institute. All rights received including rights of reproduction and use in any form or by any means, including the making of copies by any plants process, or by efectionic or menchanical does in printed, vortices, or each or receiving its owner of visual reproduction and the companion of the copyright proprieties.

The European Union - Brite EuRam III **DuraCrete - Final Technical Report** General Guidelines for Durability Design and Redesign Contract BRPR-CT95-0132 Project BE95-1347

Common Deterioration Types

- Reinforcing Steel Corrosion
- Concrete Cracking,
 Spalling, Delamination

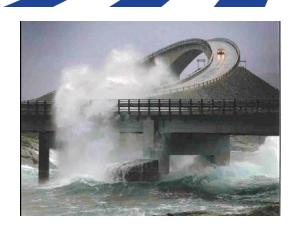


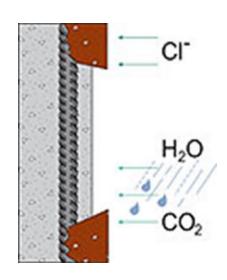
 Structural Steel Corrosion following breakdown of Protective Coating Systems



Environmental Exposure

- Chlorides from Sea Water or De-Icing Chemicals
- CO₂ from many Wet / Dry Cycles
- Temperature / Relative Humidity
- Freeze / Thaw Cycles
- Abrasion (ice action on piers, studded tires on decks)



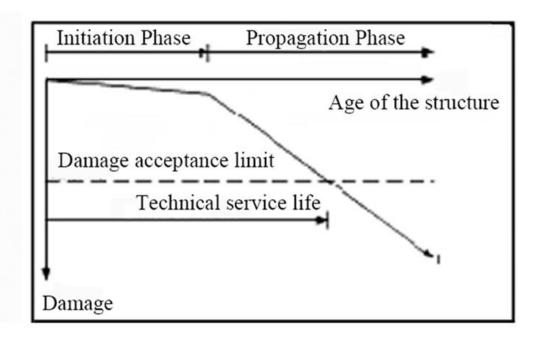


Material Resistance

- Reinforced Concrete
 - Adequate reinforcing steel cover dimension
 - High-quality concrete in the cover layer
- Structural Steel
 - Chemical composition for corrosion resistance
 - Protective Coatings

Deterioration Modeling

- Reinforcing Steel Corrosion is defined with a two-phase deterioration model
 - Initiation No visible damage is observed
 - Propagation Corrosion begins and progresses



Service life of concrete structures. A two-phase modelling of deterioration.

[Tuutti model (1982)]

Example Deterioration Model

 Chloride Ingress – Fick's 2nd Law of Diffusion for Corrosion Initiation

$$C_{crit} \geq C(x = a, t) = \frac{\mathbf{C_o} + (\mathbf{C_{s,\Delta x}} - \mathbf{C_o}) \cdot \left[1 - \text{erf}\left(\frac{\mathbf{a} - \Delta x}{2\sqrt{D_{app,C} \cdot t}}\right)\right]}{2\sqrt{D_{app,C}} \cdot \mathbf{k_t} \cdot \mathbf{A}(t)}$$

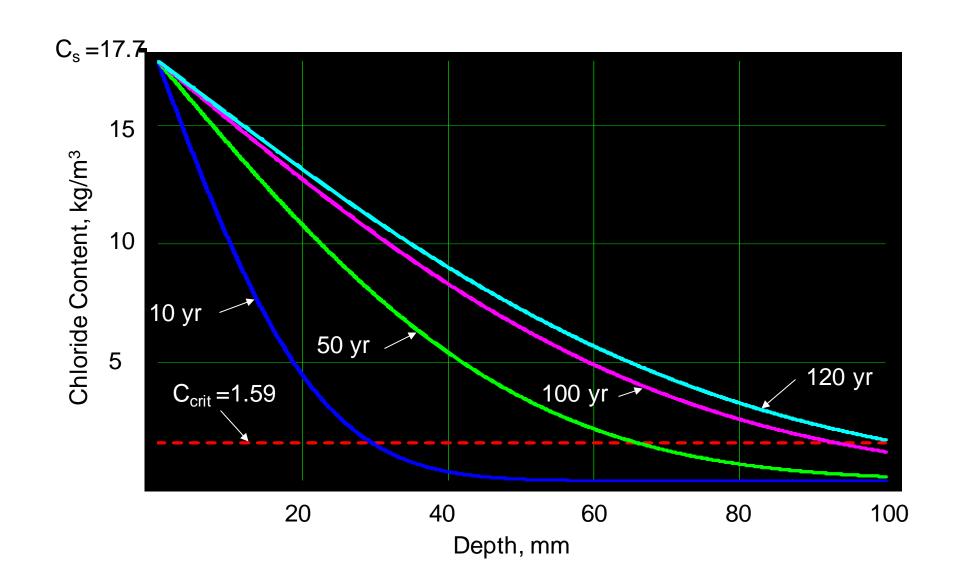
$$k_e = \exp\left(b_e\left(\frac{1}{T_{ref}} + \frac{1}{T_{real}}\right)\right)$$

$$A(t) = \left(\frac{t_o}{t}\right)^{\alpha}$$

- Red Environmental Loading
 - C_o & C_s are the <u>Chloride Background and Surface Concentrations</u>
 - T_{real} is the <u>Annual Mean Temperature</u> at the project site
- Green Material Resistance
 - $D_{RCM,0}$ is the <u>Chloride Migration Coefficient</u>, α is the <u>Aging Exponent</u>, both are functions of the concrete mix (*W/C* ratio, SCMs)
 - a is the Concrete Cover

Chloride Profiles vs. Age

constant $D_{app,c} = 15.1 \text{ mm}^2/\text{yr}$



Current Specifications

- fib Bulletin 34 Model Code for Service Life Design (2006)
- fib Model Code for Concrete Structures 2010
- ISO 16204 Durability Service Life Design of Concrete Structures (2012)
- All focus on Concrete Structures only, little available for Steel





INTERNATIONAL STANDARD

ISO 16204

Pirst edition

Durability — Service life design of concrete structures

Durabilité - Conception de la durée de vie des structures en bétor

Through-Life Management

- Integrating all stages in the life of a structure
 - Design
 - Construction
 - In-Service Maintenance & Inspection
 - Intervention (Repair & Rehabilitation)
 - Dismantling
- Future oriented toward sustainable, life-cycle thinking

Through-Life Stages

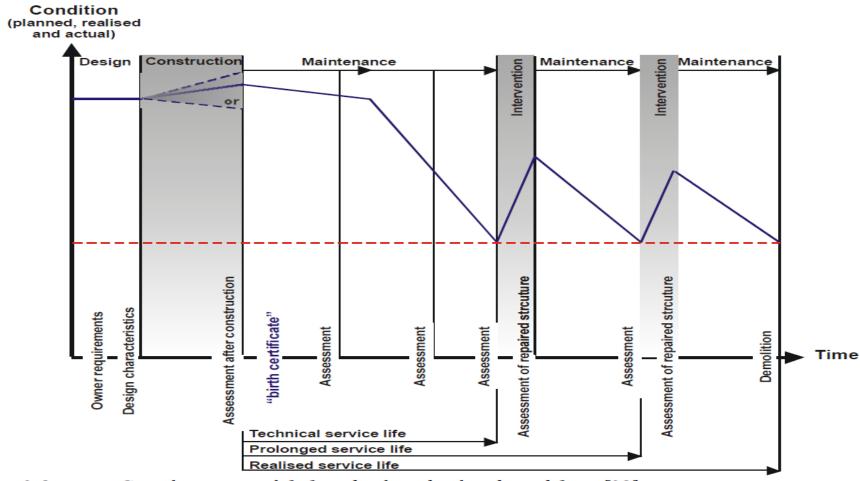


Fig. 2-1: Complete service life from birth to death, adapted from [28]

Service Life Design Strategies

Avoidance of deterioration – Strategy A

- Design based on deterioration from the environment – Strategy B
 - Full probabilistic design
 - Deemed to satisfy provisions
 - Semi-probabilistic or deterministic design
- "One size does not fit all" Multiple strategies may be used on a single bridge

Avoidance of Deterioration

- Also called the "Design-Out" approach
- Achieved by either:
 - Eliminating the environmental exposure actions
 - e.g., Use of alkali-non-reactive aggregates
 - Providing materials with resistance well beyond the requirements needed
 - e.g., Use of stainless steel reinforcement
 - Not always the most cost-effective solution

Full Probabilistic Design

- Uses mathematical models to describe observed physical deterioration behavior
- Model variables are:
 - Environmental exposure actions (demands)
 - Material resistances (capacities)
- Variables represented by mean values and distribution functions (std. deviations, etc.)
- Probabilistic, Monte-Carlo type analysis to compute level of reliability

Full Probabilistic Design

- Reliability based like that used to develop AASHTO LRFD code for structural design
- Sophisticated analysis often considered beyond the expertise of most practicing bridge engineers
- Work effort may be regarded as too time consuming for standard structures
- Has been reserved for use on large projects

Deemed to Satisfy Method

- Prescriptive approach used in most major design codes, like AASHTO LRFD sections 2.5.2.1 & 5.12
- Based on some level of past performance "Rules of Thumb"
- No mathematical deterioration modeling
- Simplistic and not quantifiable
- Lowest level of reliability

- 2.5.2.1 Durability
 - Contract documents shall call for quality materials and ... high standards of fabrication and erection.
 - Structural steel shall be self-protecting, or have longlife coating systems or cathodic protection.
- Good intention, but hardly quantifiable

- 5.12.1 Durability General
 - Concrete structures shall be designed to provide protection of the reinforcing and prestressing steel against corrosion throughout the life of the structure.
 - Special requirements that may be needed to provide durability shall be indicated in the contract documents.
- · Again, not very much guidance

- 5.12.3 Durability Concrete Cover
 - Cover for unprotected prestressing and reinforcing steel shall not be less than that specified in Table 5.12.3-1 and modified for W/C ratio...
 - Modification factors for W/C ratio shall be the following:

 - For W/C ≥ 0.5 1.2

Specified concrete cover dimensions

SECTION 5: CONCRETE STRUCTURES

Table 5.12.3-1—Cover for Unprotected Main Reinforcing Steel (in.)

| Situation | Cover (in.) |
|---------------------------------------|-------------|
| Direct exposure to salt water | 4.0 |
| Cast against earth | 3.0 |
| Coastal | 3.0 |
| Exposure to deicing salts | 2.5 |
| Deck surfaces subject to tire stud or | 2.5 |
| chain wear | |
| Exterior other than above | 2.0 |

Cover minimally related to concrete properties

Deemed to Satisfy Evaluation

- fib Commission 8 Durability
 - Used full probabilistic methods to evaluate level of reliability for deemed to satisfy code provisions for chloride ingress
 - 9 countries evaluated, including US
 - Results published in 2015



Benchmarking of deemed-tosatisfy provisions in standards



Reliability Levels

| Summary of Reliability Index, β versus Probability of Failure, P _f | | | |
|---|-------------|-----------------------------|--|
| | | | |
| | | | where $-\phi_U^{-1}(P_f)$ is defined as the inverse standard |
| P _f | Reliability | $\beta = -\phi_U^{-1}(P_f)$ | normalized distribution function |
| | | | |
| | | | Example |
| | | | |
| | | | fib Bulletin 34 Model Code for Service Life, corrosion |
| 10% | 90% | 1.3 | initiation |
| | | | Eurocode EN 1990 (service limit state calibrated for a 50 |
| 6.7% | 93.3% | 1.5 | year design life) |
| 1.0% | 99% | 2.3 | |
| 0.1% | 99.9% | 3.1 | |
| | | | AASHTO LRFD Strength I (calibrated for 75 year design |
| 0.02% | 99.98% | 3.5 | life) |
| | | | Eurocode EN 1990 (ultimate limit state calibrated for a 50 |
| 0.007% | 100% | 3.8 | year design life) |
| 50% | 50% | 0.0 | |
| | | | fib TG8.6 Deemed to Satisfy for exposure XD3 (chlorides |
| 80% | 20% | -0.8 | other than seawater) in USA - 50 year design life |

Semi-Probabilistic Design

- Uses same mathematical model as Full Probabilistic Design
- Load factors on environmental demands
- Resistance factors on material properties
- Direct solution to model equations
- Not enough data to properly determine appropriate factors and reliability level
- Method expected to be adopted by codes in the future

Service Life Designed Structures

Confederation Bridge, Canada –1997 (100 years)



Service Life Designed Structures

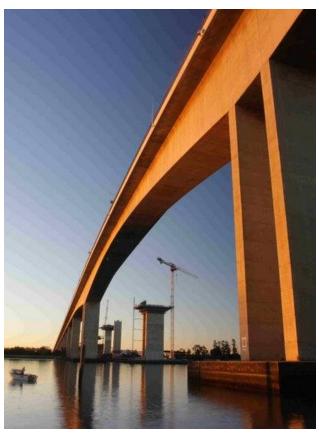
Great Belt Bridge, Denmark – 1998 (100 years)



Service Life Designed Structures

• Gateway Bridge, Brisbane – 2010 (300 years)





Service Life Designed Structures

Ohio River Bridge, KY – 2016 (100 years)



Service Life Designed Structures

Tappan Zee Bridge, NY – 2018 (100 years)



Need More Focus on These

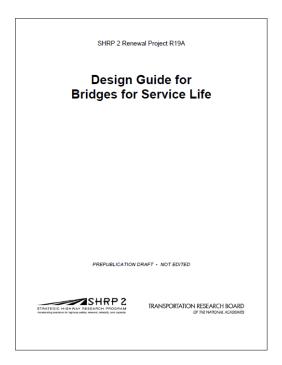
 Representing the majority of the 600,000+ bridges in the US



What's Currently Being Done

Second Strategic Highway Research Program (SHRP2)

Project R19A – Service Life Design Guide



http://www.trb.org/Main/Blurbs/168760.aspx

SHRP2 R19A Team

RESEARCH – **TRB**

IMPLEMENTATION – FHWA/AASHTO

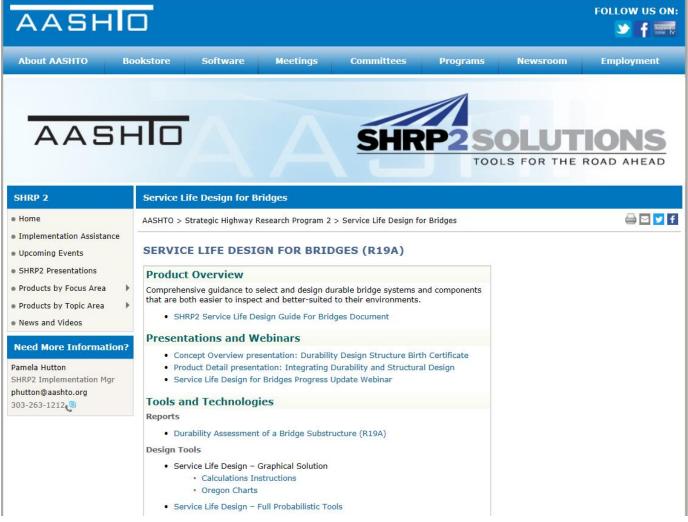
SUBJECT MATTER EXPERTS
/ LOGISTICS SME LEAD –
CH2M

TECHNICAL SME's – COWI

SHRP2 R19A Implementation Assistance Program Goals

- Promote Service Life Design concepts
 - Marketing, Outreach & Training
 - Target 15% of State DOTs by 2016
- Produce basic elements for inclusion in an AASHTO Service Life Design Guide
 - Coordinate with SCOBS and T-9
- Build a strong technical foundation
 - Develop training & reference materials
 - Lessons learned summaries

SHRP2 R19A Implementation



http://shrp2.transportation.org/Pages/ServiceLifeDesignforBridges.aspx

Who Are the Lead Agencies?



Oregon

Central Federal Lands (project in Hawaii)





Who Are the Lead Agencies?

Dubugu

IOWA

Dec Moines

Dec Moines

Dec Moines

Pennsylvania



Virginia

Virginia

Virginia

Roanoke

Virginia

Roanoke

Virginia

Roanoke

Virginia

Roanoke

Virginia

Roanoke

Virginia

Roanoke

Roan

R19A IAP Funding

 State Agencies were awarded \$150,000 each as Lead Adopters

FHWA CFL was awarded \$75,000

 Funding for technical assistance from the SME team is through SHRP2, and <u>NOT</u> part of agency awards

R19A Next Steps

Round 7 Implementation Assistance

\$500,000 in Lead Adopter awards made available

 2 Applications received on April 30, 2016 are currently being reviewed

Future Research

- AASHTO T-9 Bridge Preservation Technical Committee sponsoring NCHRP Research Project 12-108 (Pending)
- Uniform Service Life Design Guide Specification
 - Conduct Literature Review
 - Synthesize Gaps in Current Practice
 - Develop a Methodology considering:
 - Multiple Analysis Methods
 - Deterioration Processes and Exposure Zones and Loads
 - Service Life Target Based on Functional Requirements
 - Selection of Alternative Designs to Achieve Target Service Life
 - Evaluate Effectiveness of Design, Construction, Inspection Strategies and Management Practices
 - Produce Report and Guide Specification

Summary

- Durability or Service Life Design is:
 - A design approach to resist deterioration caused by environmental actions
- Design Guides/Codes are available:
 - fib Bulletin 34 Model Code for Service Life
 Design
- Current implementation
 - SHRP2 R19A projects (FHWA CFL, IA, OR, PA, VA)
- AASHTO T-9 Initiated Research
 - NCHRP 12-108 Uniform Service Life Design Guide

Questions?

Patricia Bush, AASHTO Program Manager for Engineering pbush@aashto.org

Subject Matter Expert Team:

Mike Bartholomew, *CH2M*mike.bartholomew@ch2m.com

Anne-Marie Langlois, COWI North America amln@cowi.com