

# Advanced Methods to Identify Asphalt Pavement Delamination (R06D) Ground Penetrating Radar (GPR) New Mexico DOT

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U.S. Department of Transportation  
Federal Highway Administration

AMERICAN ASSOCIATION  
OF STATE HIGHWAY AND  
TRANSPORTATION OFFICIALS

AASHTO

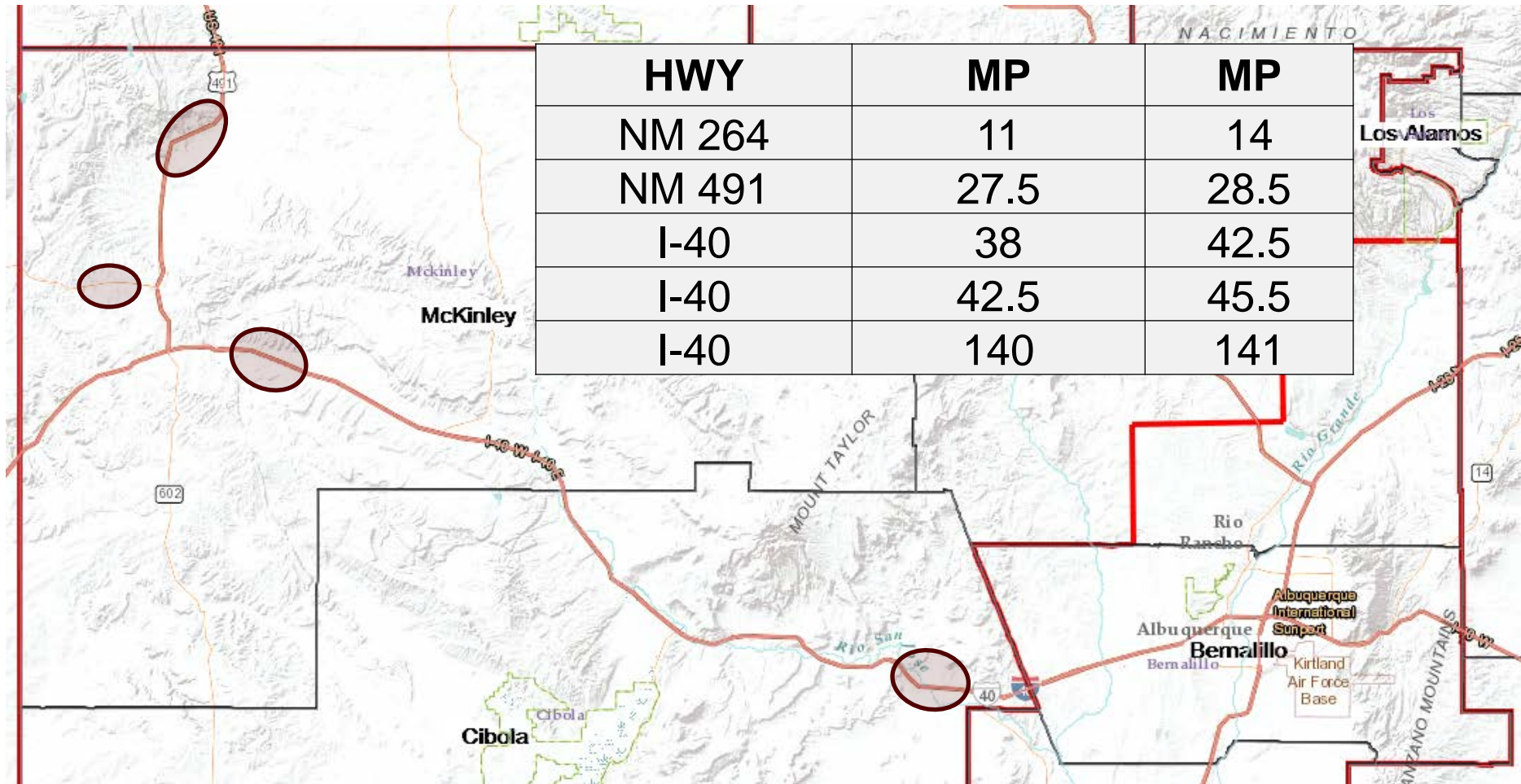
# Outline

- System Comparison TTI 1 GHz, GSSI 2 GHz and 3D Radar
  - Dr. Hayat's Study for automated analysis
- Test Site Data Analysis TTI & 3D Radar comparison
  - Field Testing
- Other testing
  - Bridge Deck
  - Concrete Pavement
- Conclusions and Recommendations

# GPR – Comparison to TTI & NM Systems



# GPR Test Locations – New Mexico



# Comparison Criteria

1. Ease of set-up, storage and operation
2. Quality of Data
3. Data storage
4. RFI or cell tower interference
5. Ease of data analysis
6. Best Pavement Applications
7. Ability to estimate Dielectric values
8. Data analysis time for detecting anomalies
9. Data analysis time for pavement layer thickness
10. Level of experience needed to analyze data
11. Ability to export files for use in other programs such as Excel
12. Reports
13. Cost
14. Ease of updating collection software, analysis software and system
15. Future applications
16. Other Capabilities
17. Synchronized Video (to assist with identifying defect locations)

# Ease of Set-up, storage & operation

High Definition  
Video Camera  
w/GPS

## TTI 1GHz System



GPR Antenna

TTI High  
Definition Video  
Camera w/GPS

## 3D Radar System



GPR Antenna

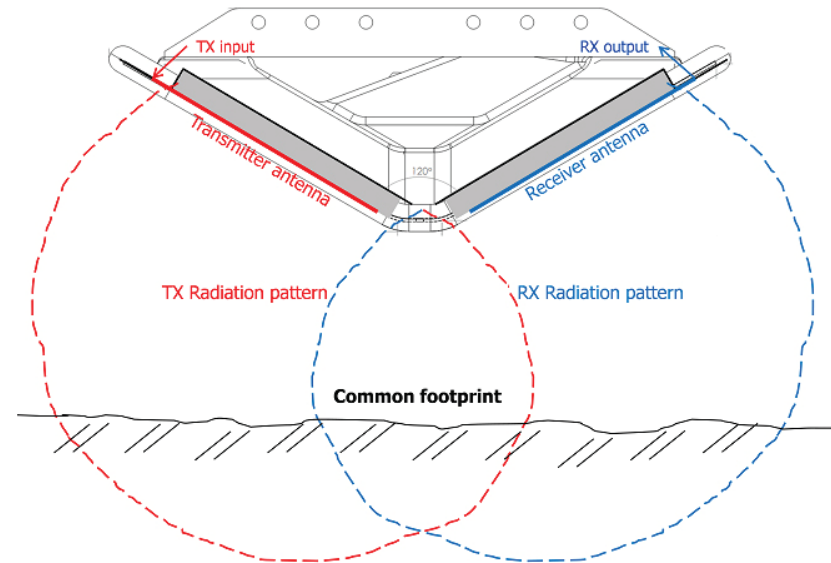
## NM 2GHz GSSI System



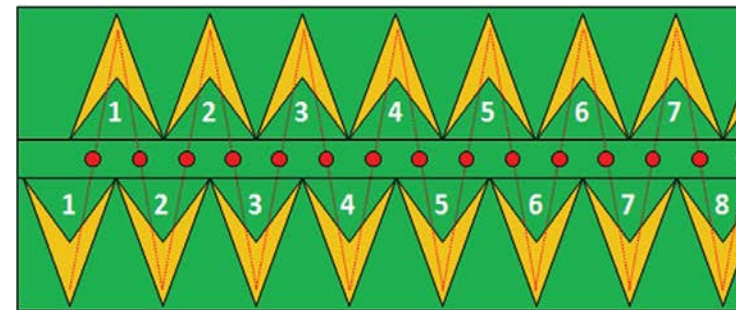
# GPR – 3D Radar

## 3D-Radar Antenna Specifications

Technical Specifications	DX1821 Antenna
Width	5.9 ft
Frequency Range	200-3000 MHz
Number Of Channels	21
Channel Spacing	3 in
Effective Scan	5.2 ft
Direct Wave Suppression	> 50 dB
Polarization	Linear (in-line direction)
Size	5.9'x1.9'x0.7'
Weight	61.7 lbs



## Antenna Configuration



## Example Scan Pattern

# Collection Settings 3D Radar

## 3D-Radar Collection Settings

Pavement Surface	<sup>2</sup> Trigger Spacing		Time Window	Dwell Time	Max Speed
	(in)	(cm)	(ns)	(us)	(mph)
<b>Concrete &amp; BRG</b>	3.0	7.6	50	0.6	43.5
<sup>1</sup> concrete/flexible	6.0	15.5	50	0.6	89
<b>Flexible</b>	12.0	30.5	50	0.6	175

1. Use for concrete pavement when need to test at >45mph; use for flexible pavement when closer spacing is needed;
2. Trigger Spacing can be increased to 36" in order to save data storage and still provide adequate network level data. If spacing is adjusted, use multiples of 3".

Note: Collection settings are preliminary and final recommendations are still under review.



# Data & Analysis

System	TTI	NM	3D
Data Quality	High Resolution	Med-High Res	Low Resolution
Storage (ex)	50,030 kb	274,948 kb	4,192,833 kb
RF (Interference)	None	Yes	Yes
Experience Needed	Basic	Basic	High Level due to filters
Software Ease of Use	Intuitive/Simple minimal training	Training needed	Training needed
Ability to Export & Report	Basic files to Excel	Basic files to Excel	Difficult
Calculate Dielectric	Yes, easy and quick	Yes, time consuming	No
Time (min) for Analysis anomalies   thickness	11:30   6:11	11:30   23:57	20:30   32:42

# Data Analysis

## Dr. Hayat's Study

- NMDOT contracted with Dr. Majeed Hayat to investigate an algorithm for distress detection based on GPR DATA.
  - Learn what things may indicate deterioration
  - Develop coefficients that can indicate deterioration
  - Compare results from coefficient analysis with ground truth data
  - Development of user interface
  - Determination of level of effort to use with 3D Radar
  - Further research to determine exact flagging thresholds are needed (currently using a statistical method to indicate what level of coefficient changes constitute deterioration)

# Assumptions

## Dr. Hayat's Study

- Deterioration will be over a finite/small area i.e., not throughout the whole pavement section
- There should be changes visible between waves in adjacent GPR scans to indicate deterioration
- Stripping should show up as a “sudden” negative amplitude wave
- Sometimes positive waves that “suddenly” appear can be indicative of water in deterioration areas

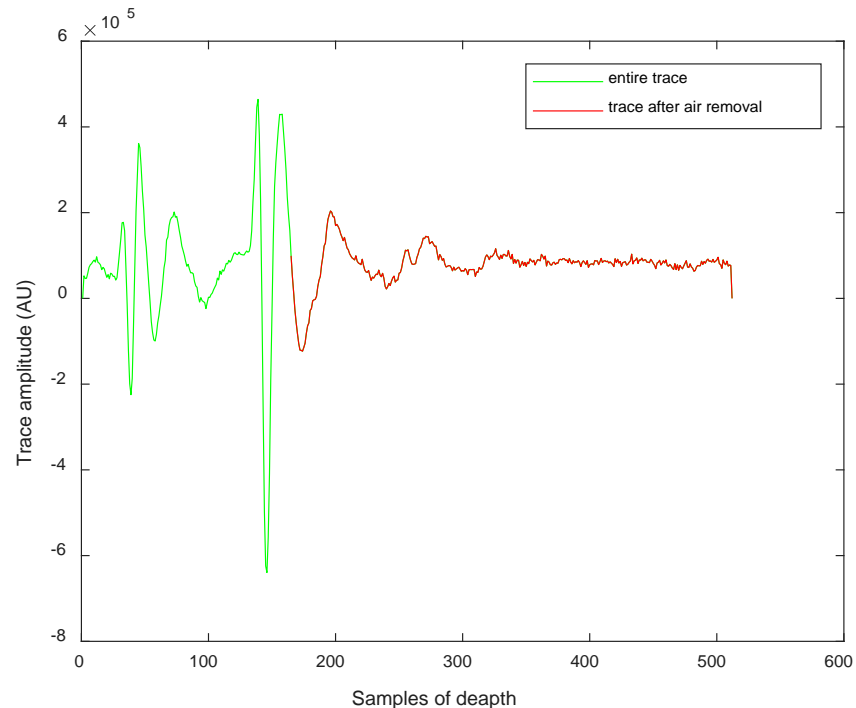
# Pre-processing removal of air

## Dr. Hayat's Study

- Only reflections that result from interfaces AFTER the air-to-asphalt interface are used in the analysis

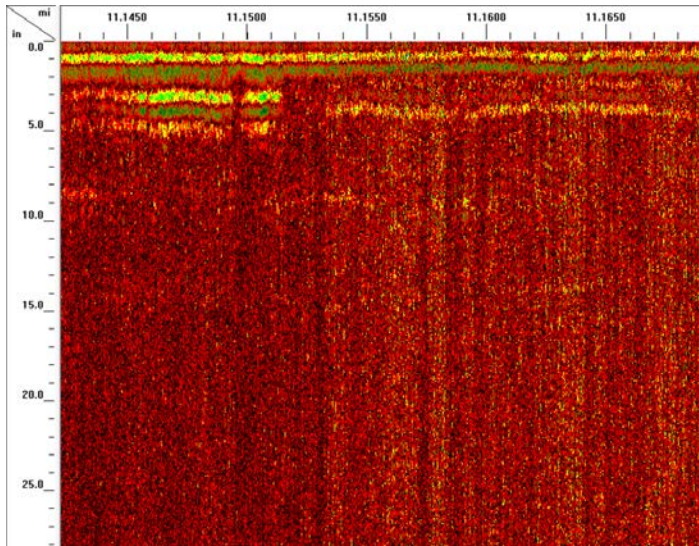
**In the example shown (I40 WB), sample 180 to 512 are maintained and the earlier samples are discarded**

**The starting point is selected by inspection**



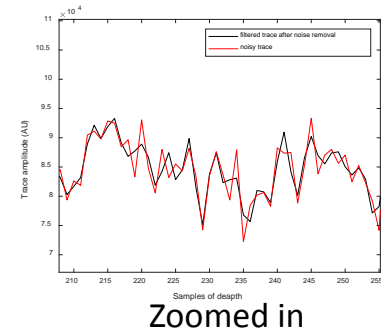
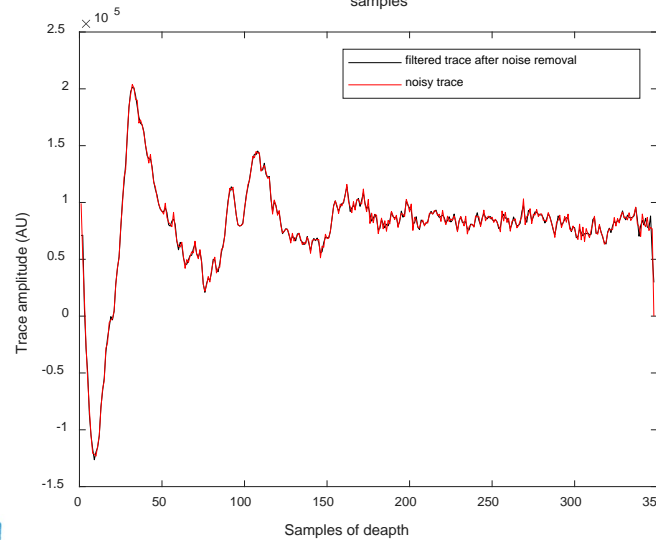
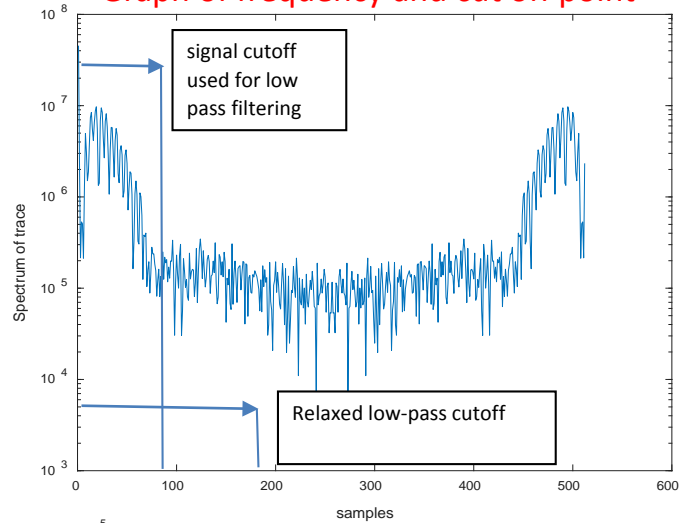
# Pre-processing Filtering

## Dr. Hayat's Study



Graph of filtered vs. unfiltered wave from I-40

Graph of frequency and cut off point

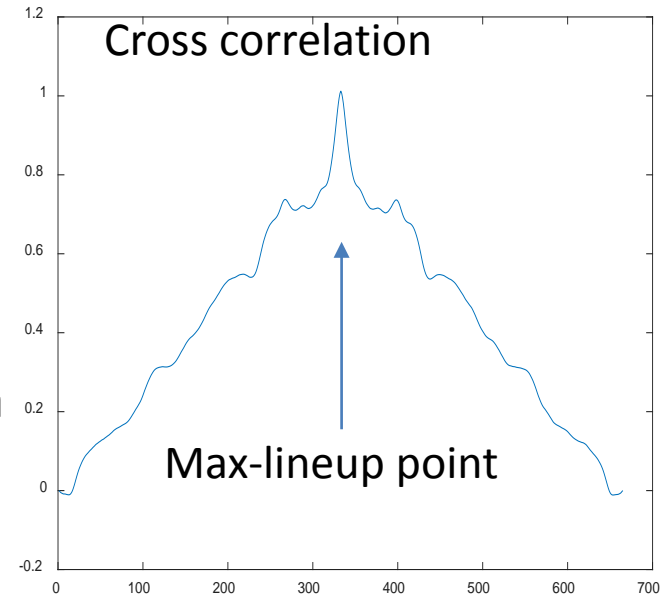
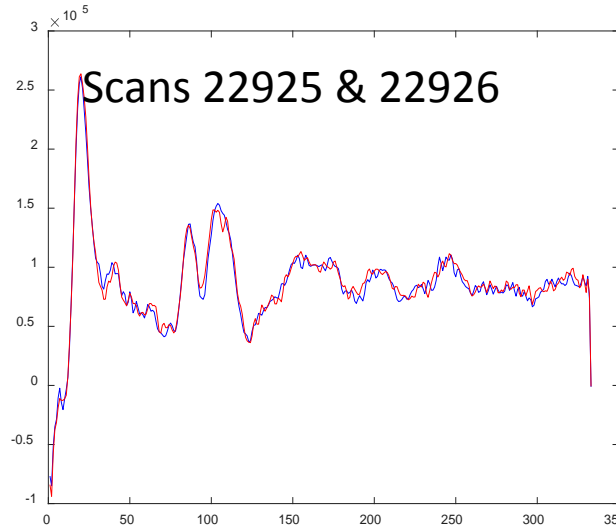


# Uniformity coefficients

## Dr. Hayat's Study

### Maximum cross correlation calculation:

- Every scan is shifted relative to the next scan, the two scans are multiplied, and integral is calculated
- Maximum area is obtained when maximum lineup is achieved
- This maximum integral is the cross correlation

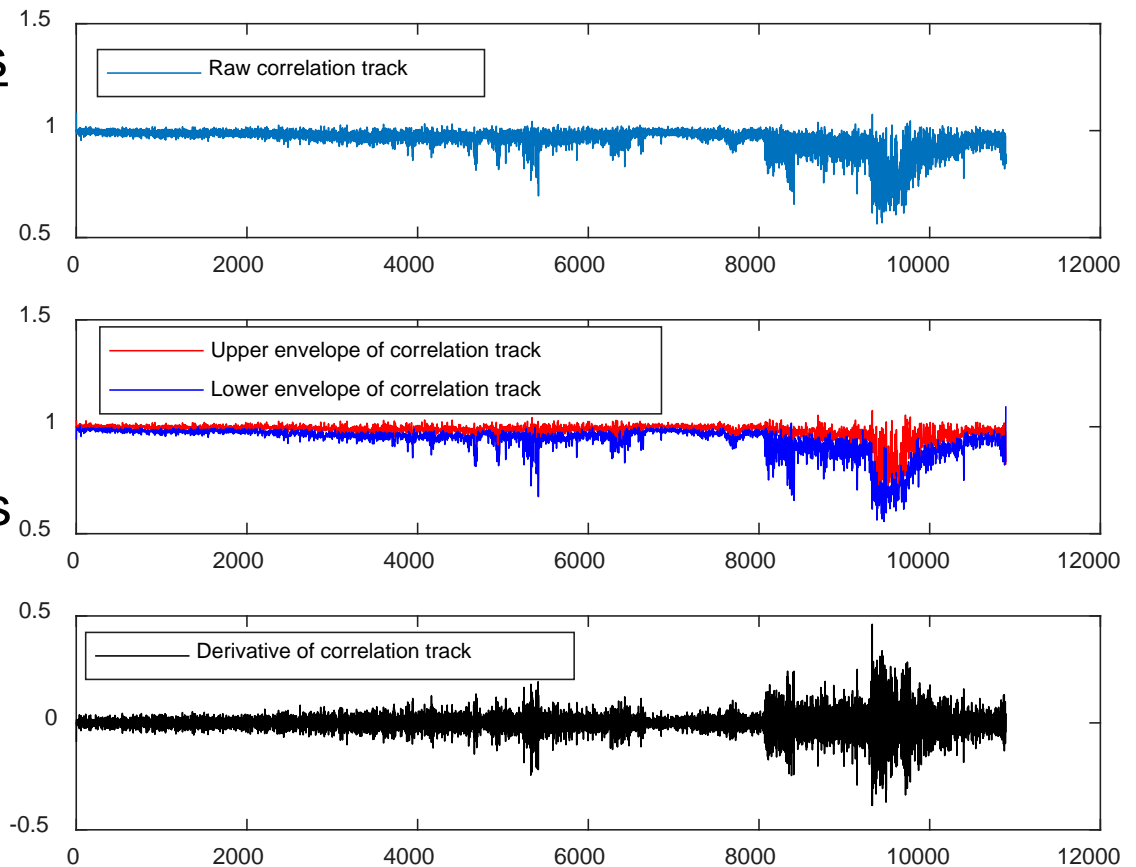


# Uniformity coefficients

## Dr. Hayat's Study

We can define a family of 3 correlation-based coefficients

- Raw correlation coefficient track (as described on the previous slide)
- Derivative of correlation coefficient track (with respect to scan number)
- Upper and lower envelopes of the correlation track



# Positive/negative peak family

## Dr. Hayat's Study

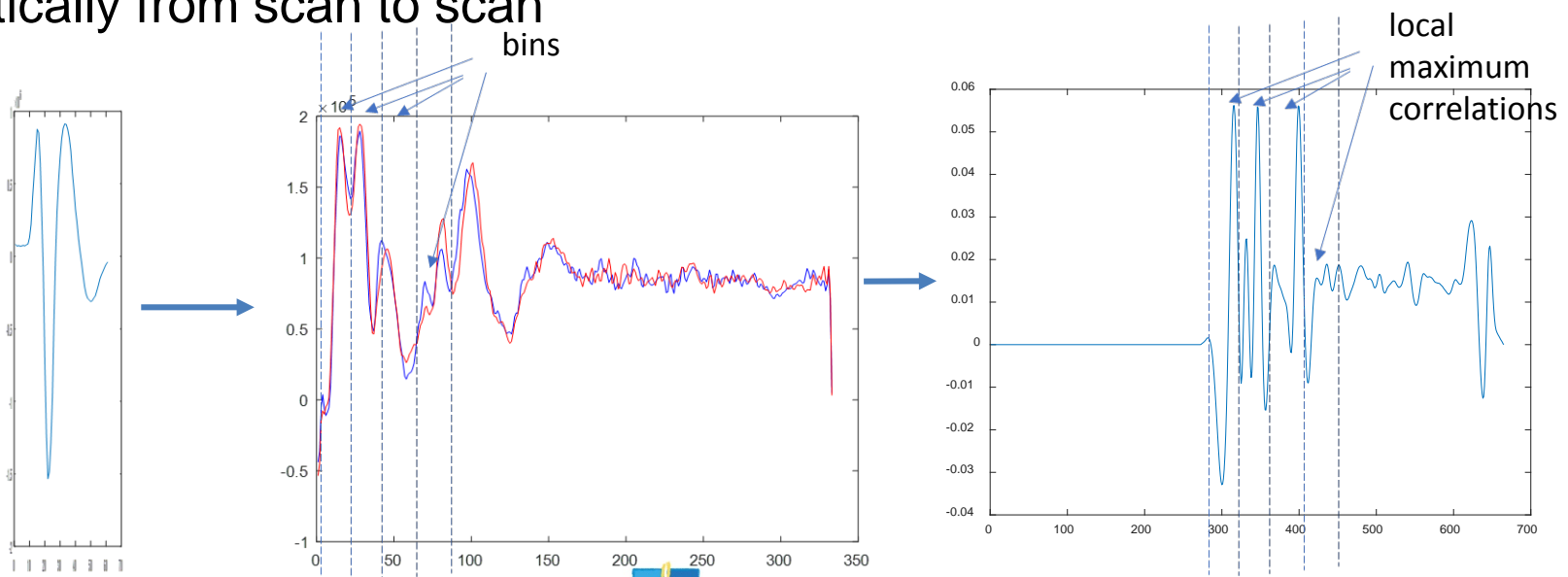
- Here, we look for the sudden presence of a positive peak or negative peak in the trace, where a “peak” is defined by the shape of the pulse from a metal plate reflection



# Negative peak

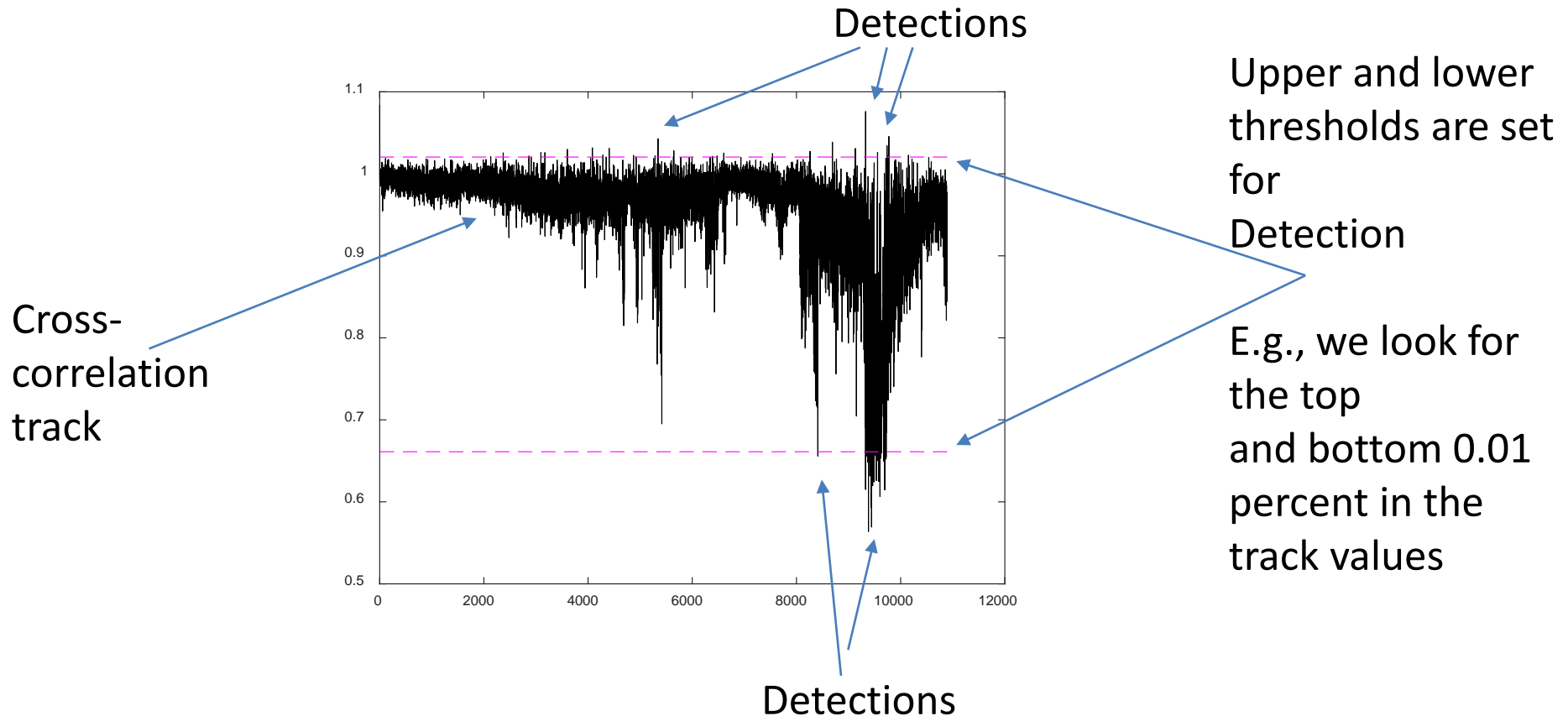
## Dr. Hayat's Study

- We divide each scan into bins of length 20 samples (see middle figure)
- We look for the presence of a negative peak (left figure) in each bin
  - This is done by cross correlating a pulse (left) with each trace (middle)
  - The peak of the cross correlation over each bin is indicative of the presence of a negative peak in that bin (right)
- We announce a negative-peak detection if *any* of the local maxima changes drastically from scan to scan



# Detection using the metric tracks

## Dr. Hayat's Study

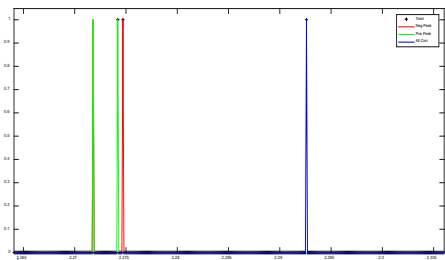
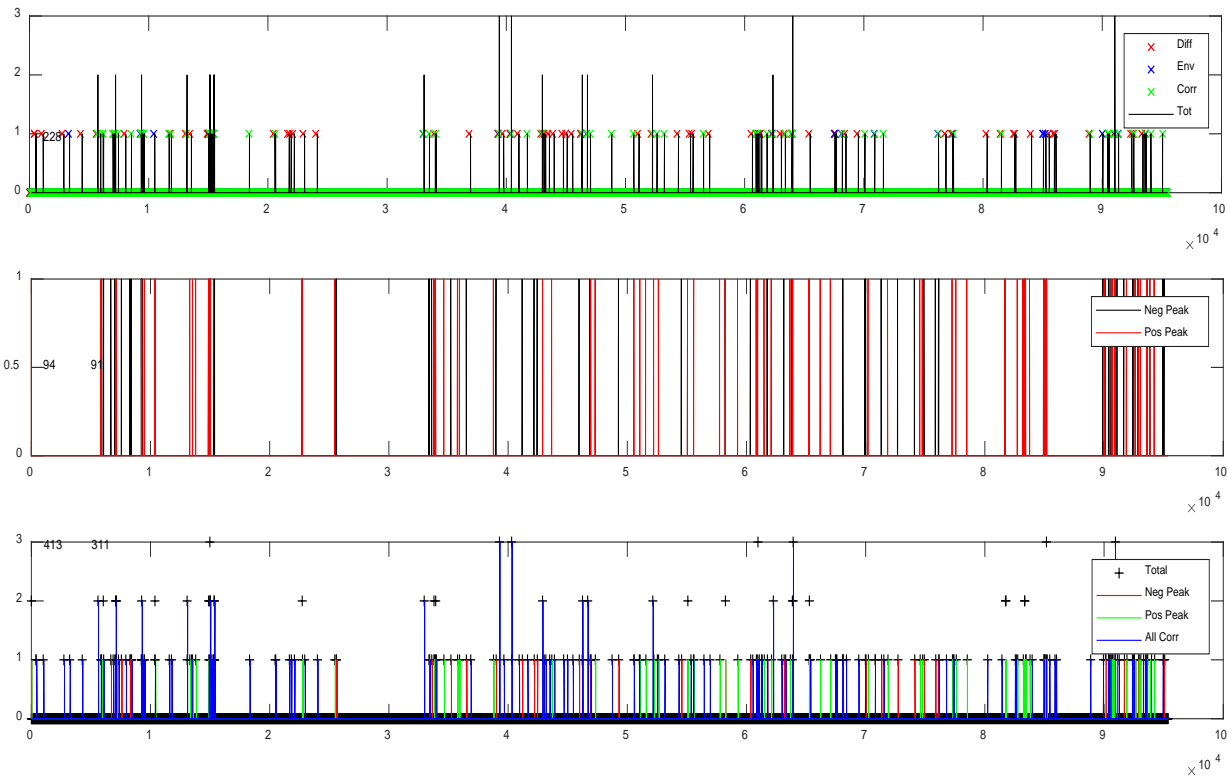


# Detection using the metric tracks

## Dr. Hayat's Study

311 detected scans (no double counting)

- Top: detections resulting from changes in uniformity metrics over scans
- Middle: detections resulting from changes negative and positive peak metrics over scans
- Bottom: All together

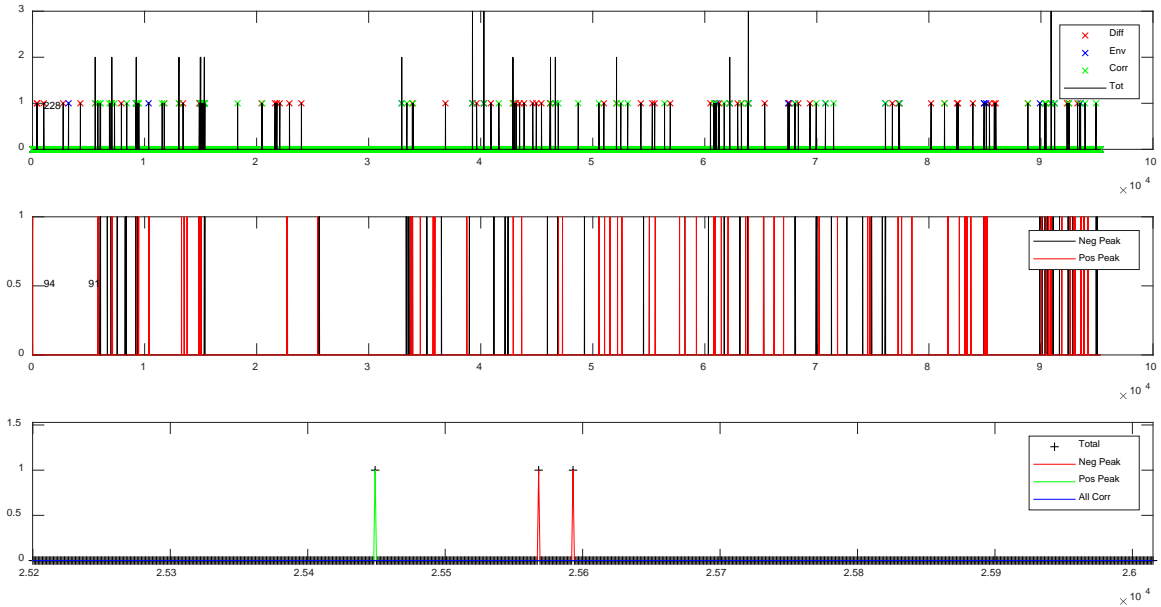


22650

23050

# Old NM264 example

## Dr. Hayat's Study



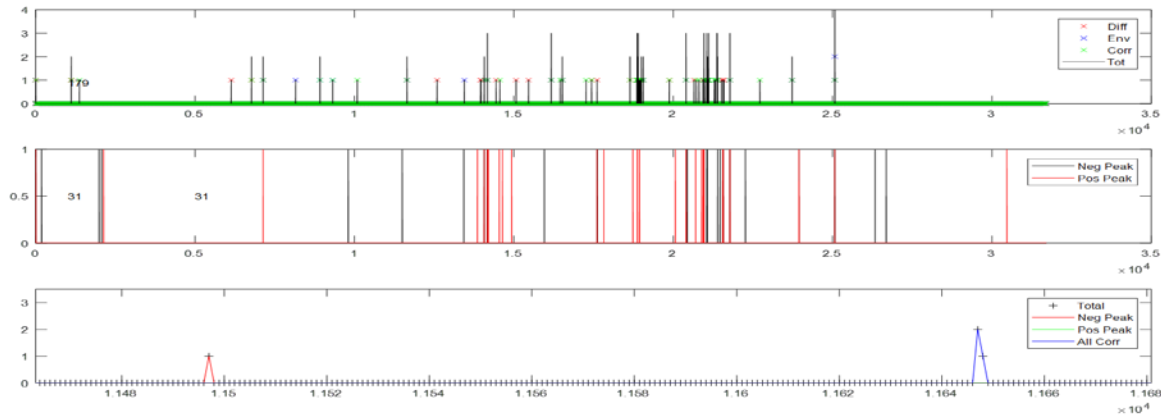
Scan ~ 25,500

Scan ~ 25,800

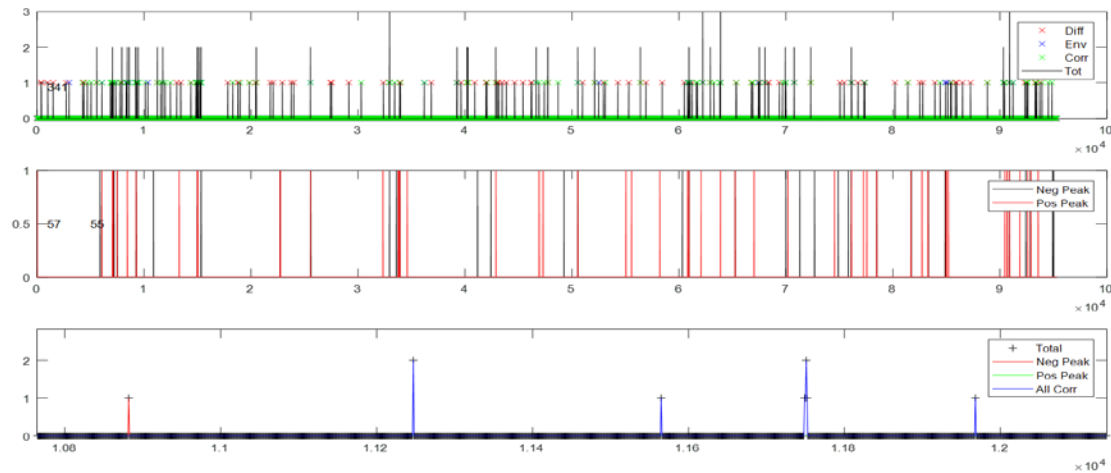
# New NM 264 Example

## Dr. Hayat's Study

2017 scan



2015 scan



Scan ~ 11,600

# Table of % error for old NM264

## Dr. Hayat's Study

Direction	MP	core	scan #	Trigger	correct?
EB	10.735	Failure/patch	22,900	Yes	yes
EB	10.745	Debonding	23,200	No	No-but debonding
EB	10.82	Fatigue failure	25,500	yes	Yes
EB	10.825	Stripping	25,800	yes	Yes
EB	11.32	Deteriorated completely	41,200	No	No-GPR limitation?
EB	12.67	Intact	83,700	No	yes

# Table of % error for new NM 264

## Dr. Hayat's Study

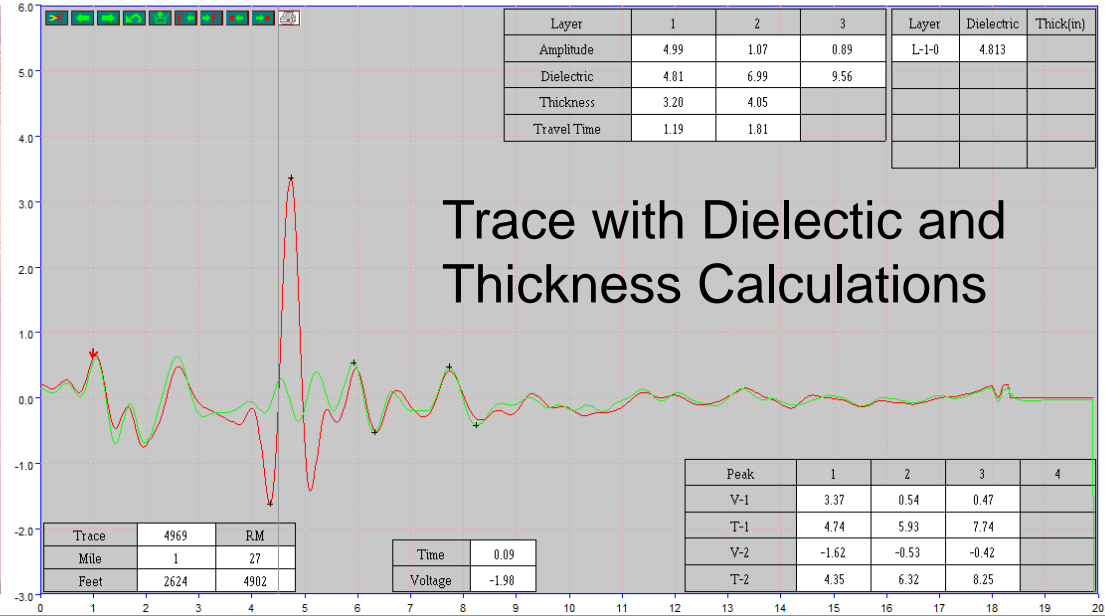
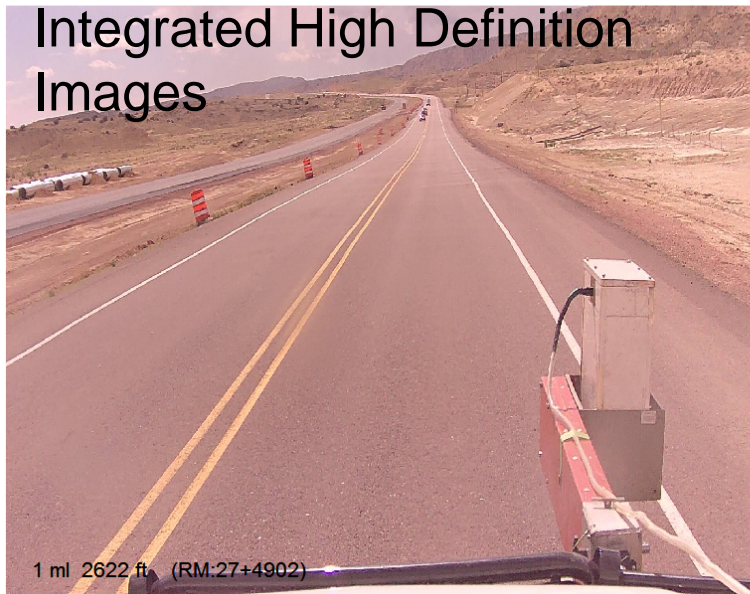
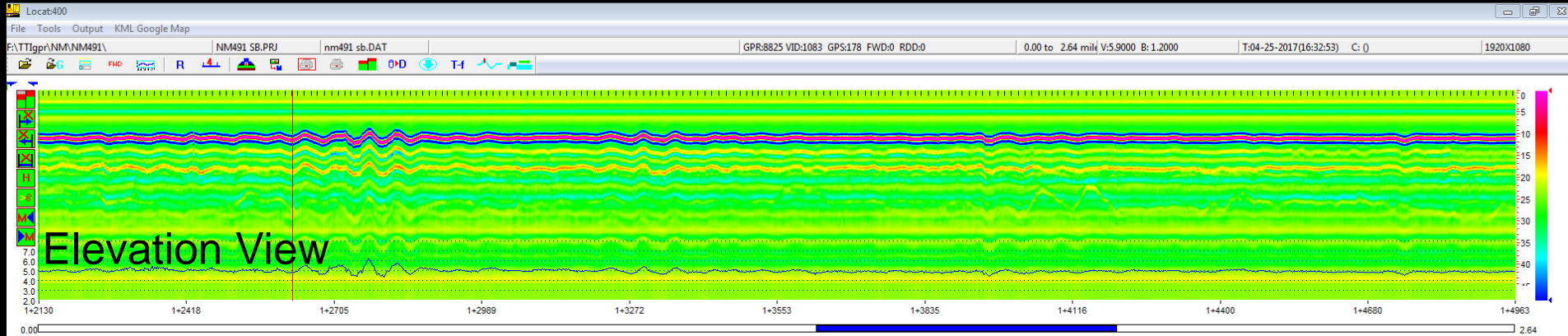
Direction	MP	core	scan #	Trigger scan #	correct?
WB	11.7	intact	24200		yes
WB	12.976	intact	10850		yes
WB	12.1	patch area	20000	20035	yes
WB	11.9	high air voids	22100	22103	yes
WB	11.3	intact	28400		yes
WB	11.2	intact	29450		yes
EB	11.1	high air voids	1054	1100	yes
EB	12.1	stripping	11600	11647	yes

# General Comparison

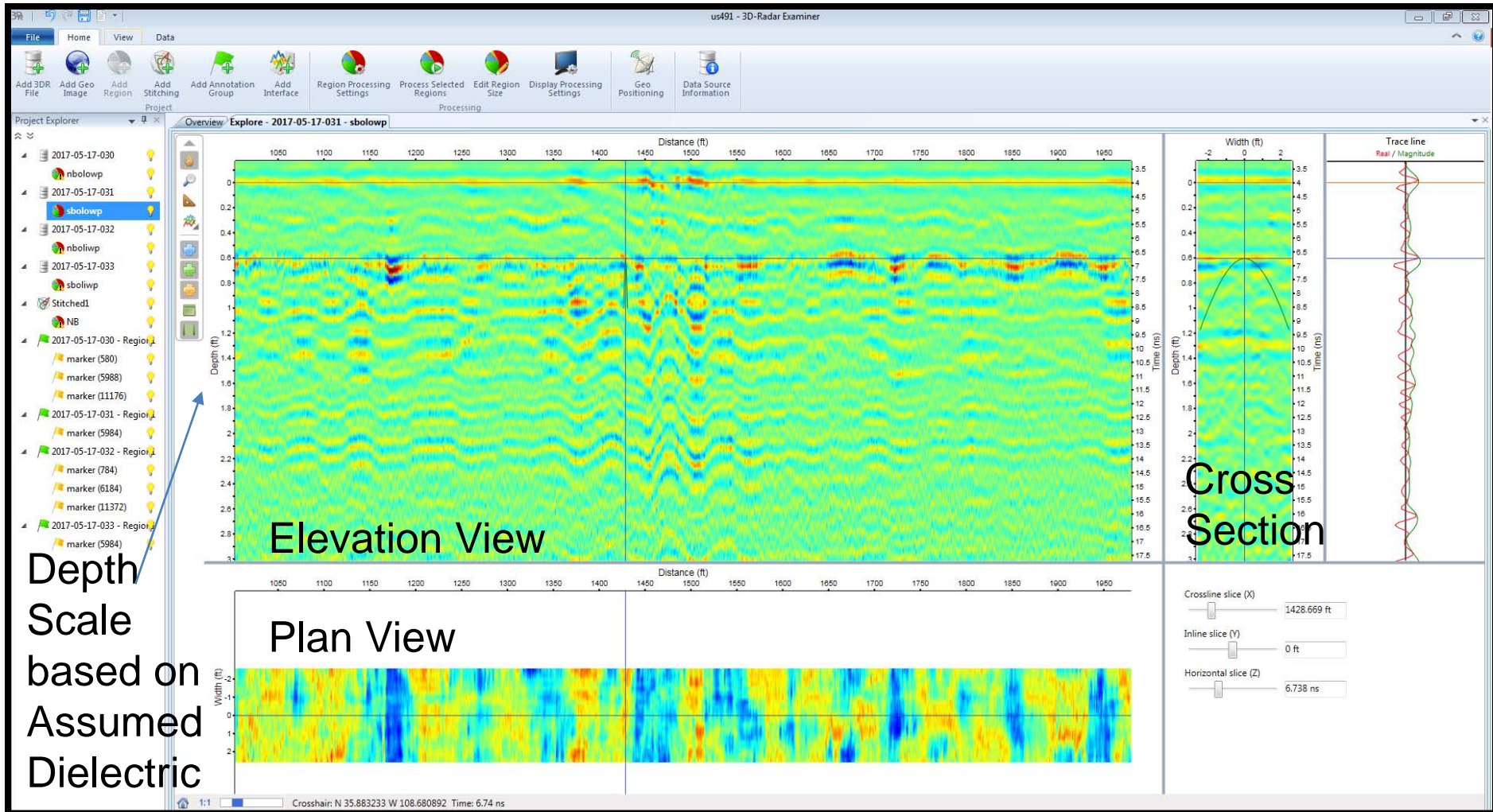
System	TTI	NM	3D
Cost	No Longer Available	~\$175,000	~\$230,000
Updates	n/a	New Equipment	Can Update
Collection	All Similar	All Similar	All Similar
Analysis	Easy	Moderate	Difficult
Future	n/a	n/a	Calculate Dielectric & depths



# TTI – PaveCheck Software



# 3D Radar Examiner Software



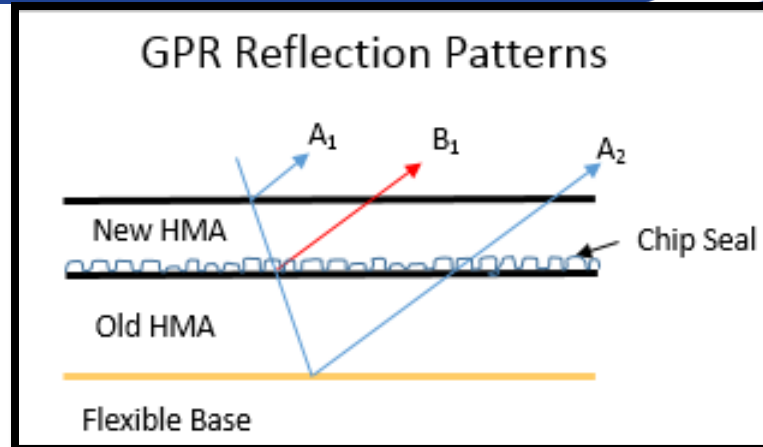
Depth Scale based on Assumed Dielectric

Elevation View

Plan View

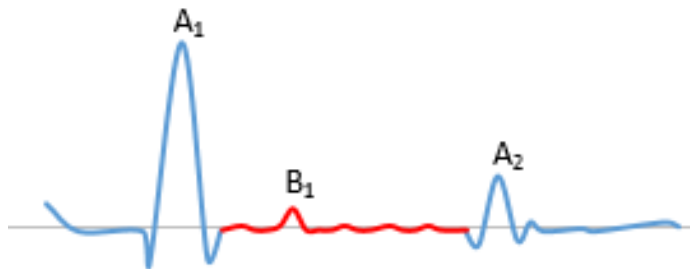
Cross Section

# Trace Patterns



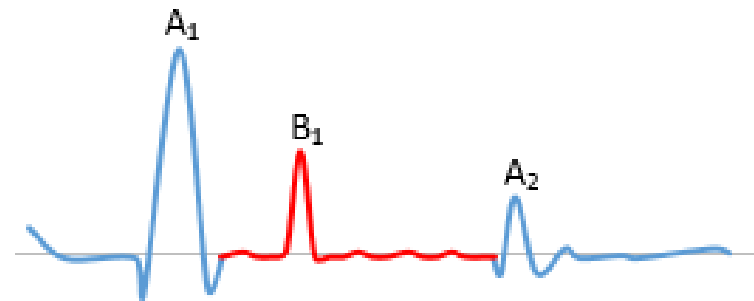
## Case-1: Normal

B<sub>1</sub> Positive, very small reflection  
Little dielectric contrast  
between new and old HMA

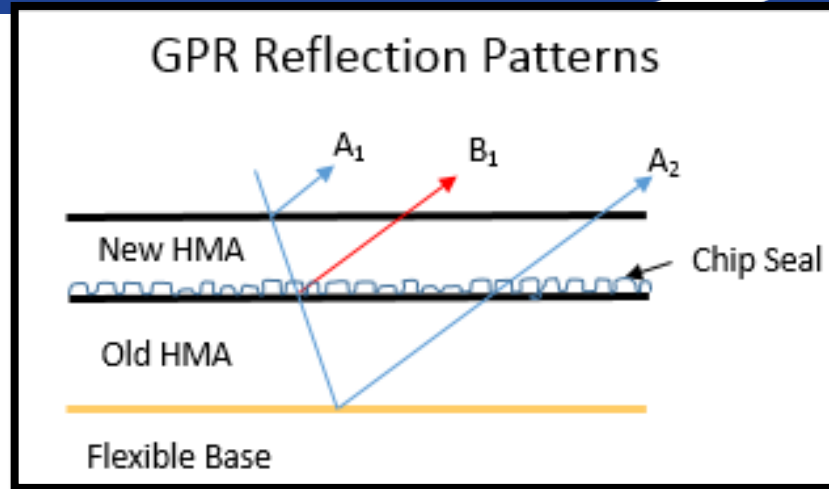


## Case-2: Trapped Moisture

B<sub>1</sub> Large Positive reflection  
A<sub>2</sub> reflection is smaller than B<sub>1</sub>

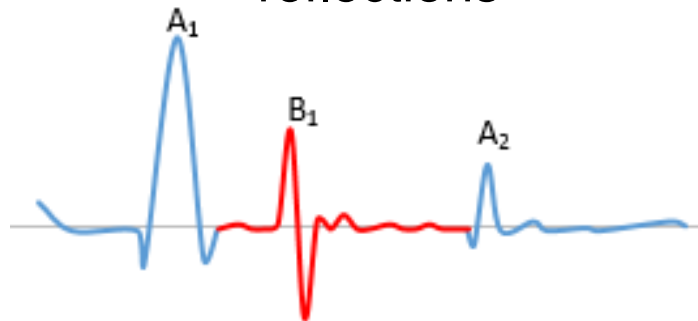


# Trace Patterns



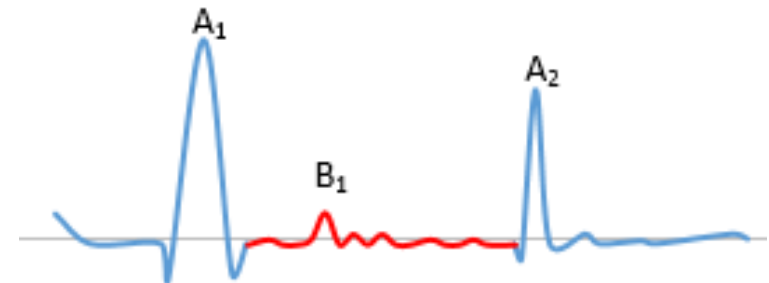
## Case-3:

Thin Layer of Moisture on Top of Seal  
B<sub>1</sub> Overlapping Positive and Negative reflections

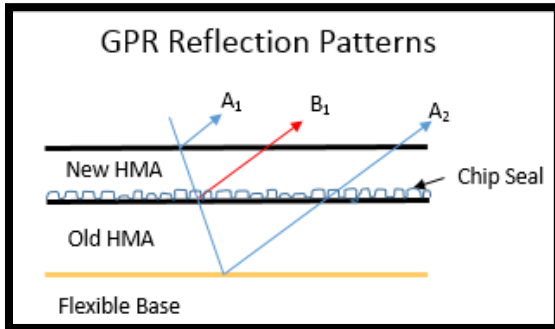


## Case-4:

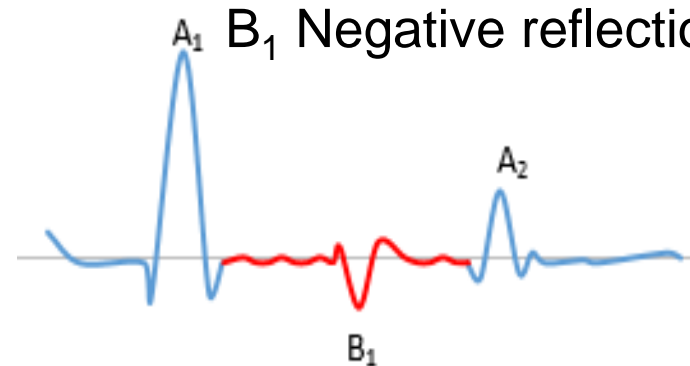
Moisture Trapped in Base Layer  
A<sub>2</sub> Reflection increases significantly



# Trace Patterns



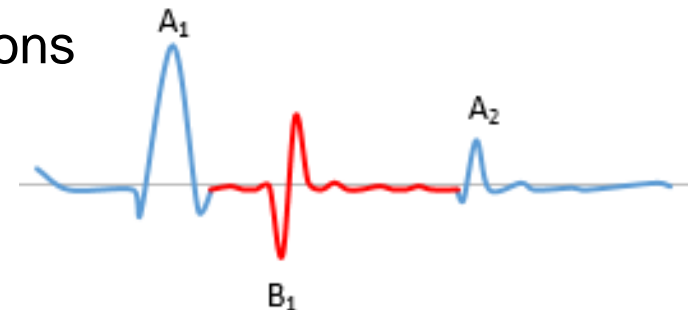
## Case-5: Lower HMA Severely Deteriorated-Stripping



Note: large negative reflections may be high void area, stripping or debonding

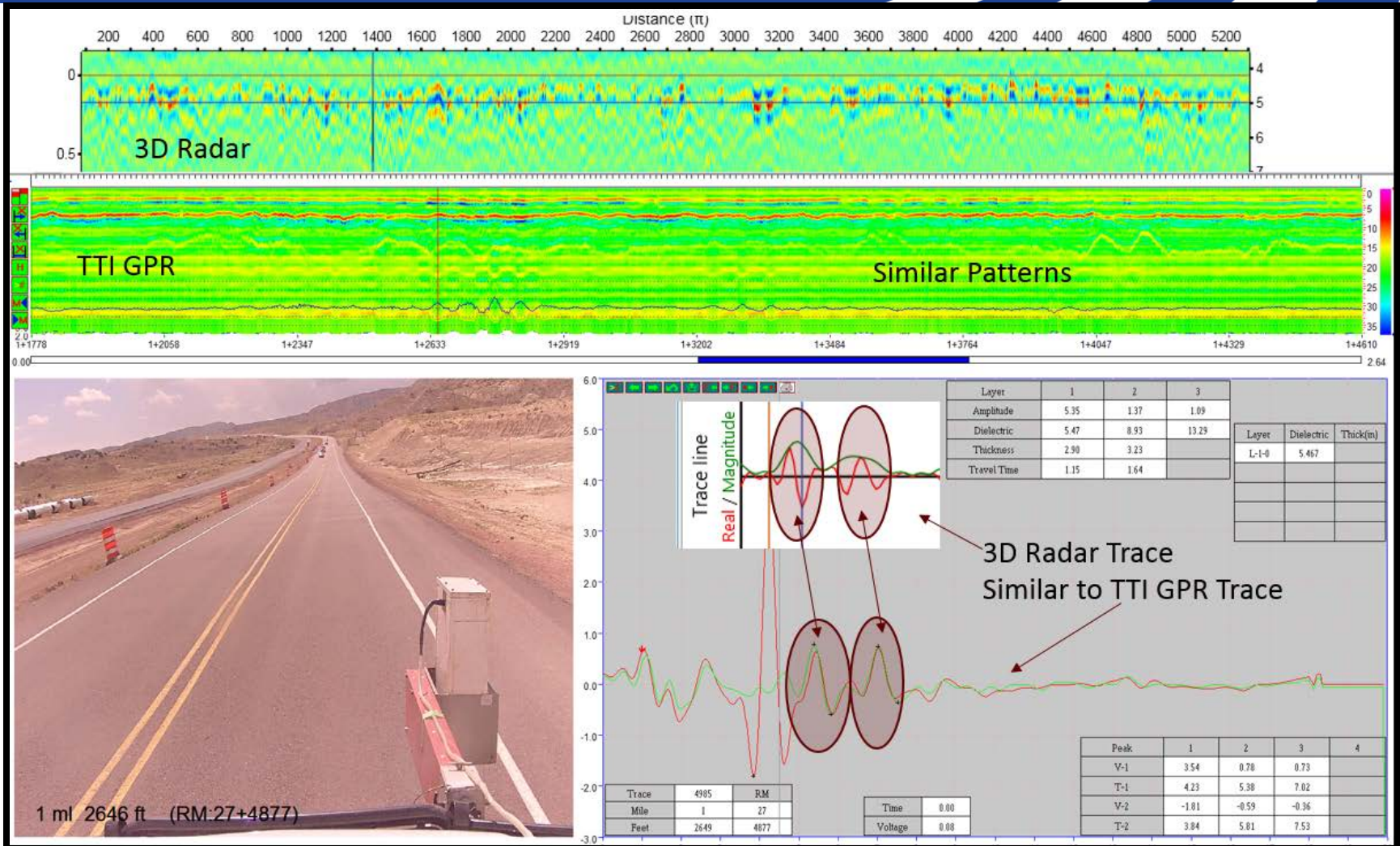
## Case-6: Stripped Layer to Good HMA

B<sub>1</sub> Overlapping Negative and Positive reflections

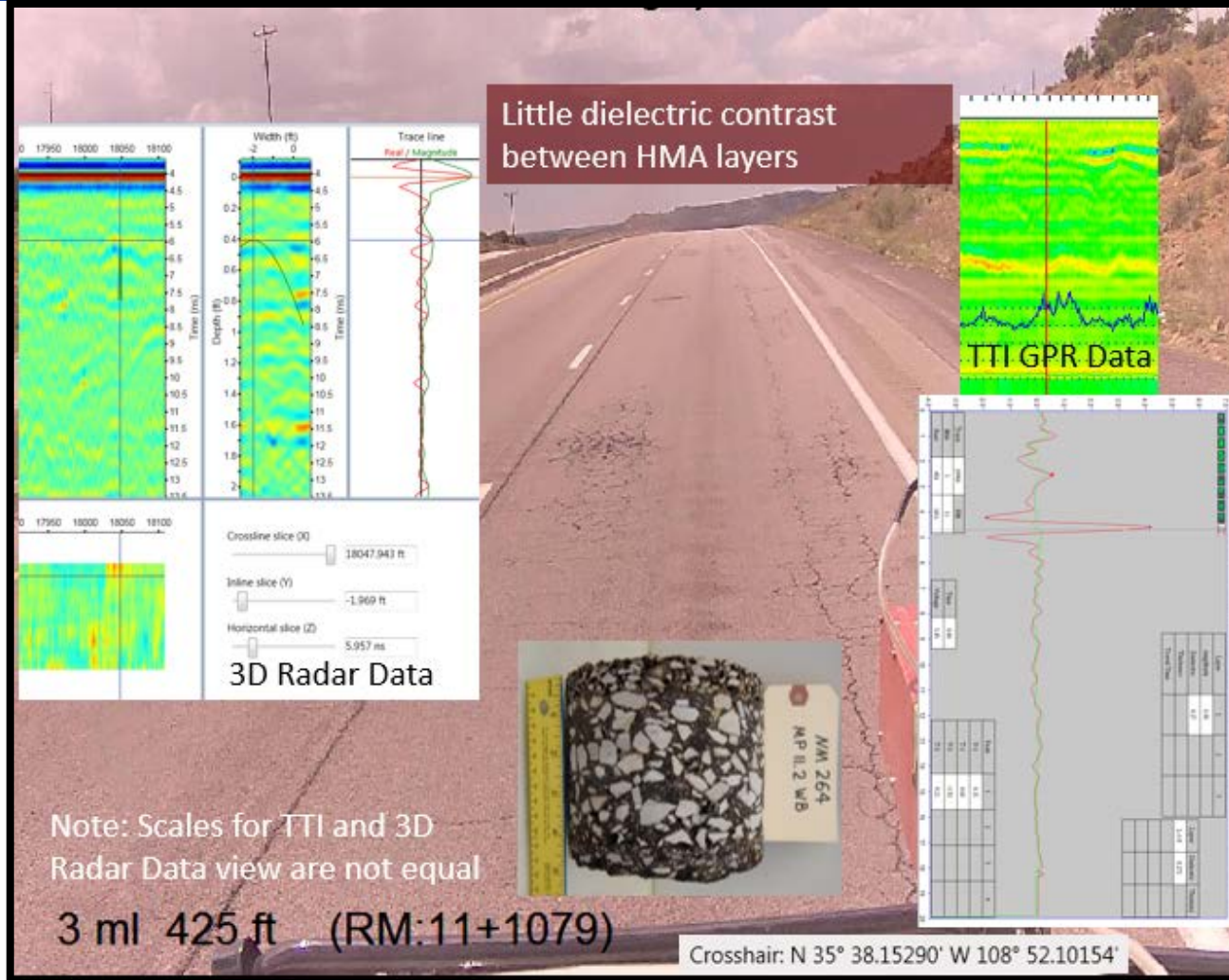


Note: Pattern may also appear when a Light weight aggregate Chip Seal is between good layers. No moisture present.

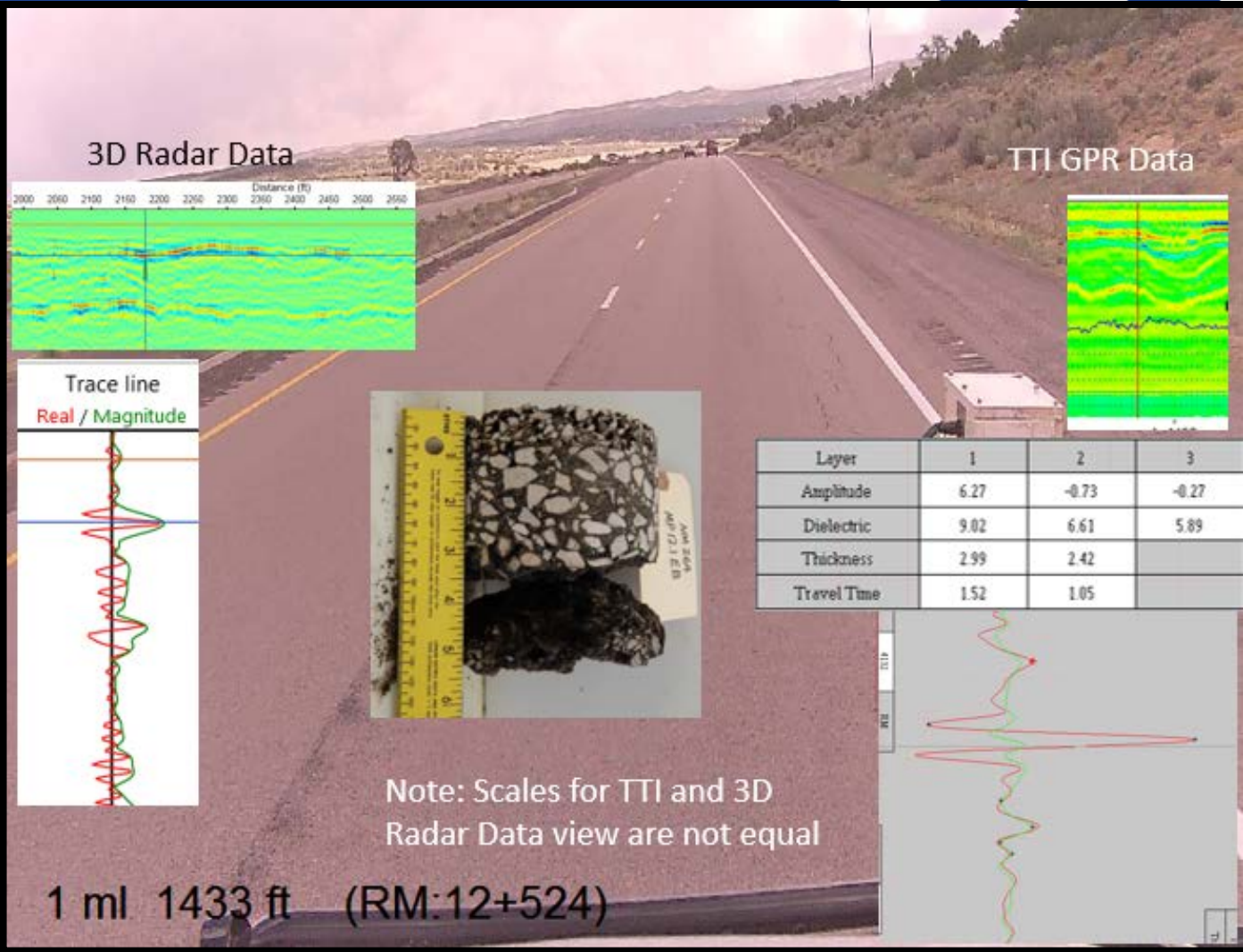
# Overview



# Comparison – District 6

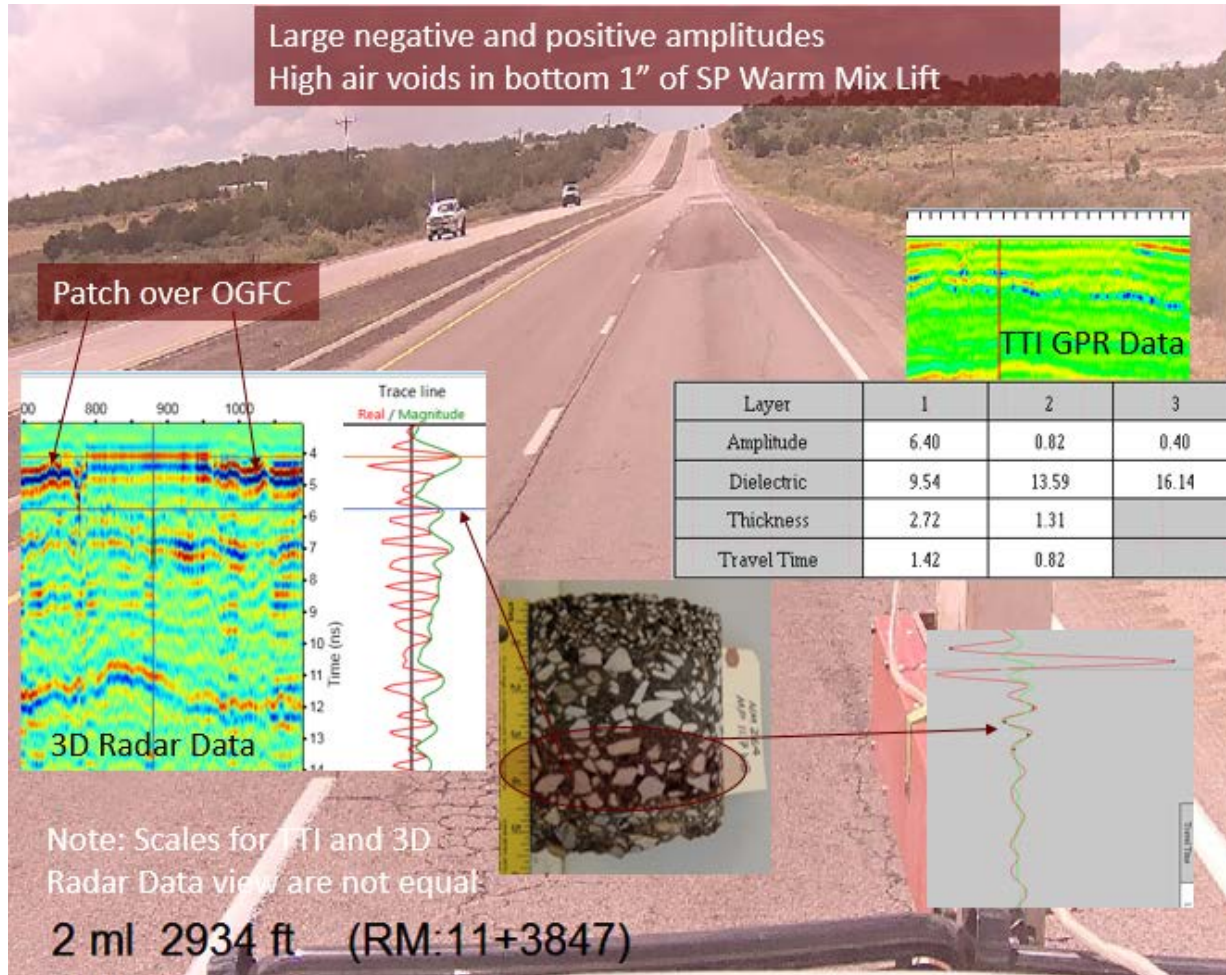


# Comparison - District 6

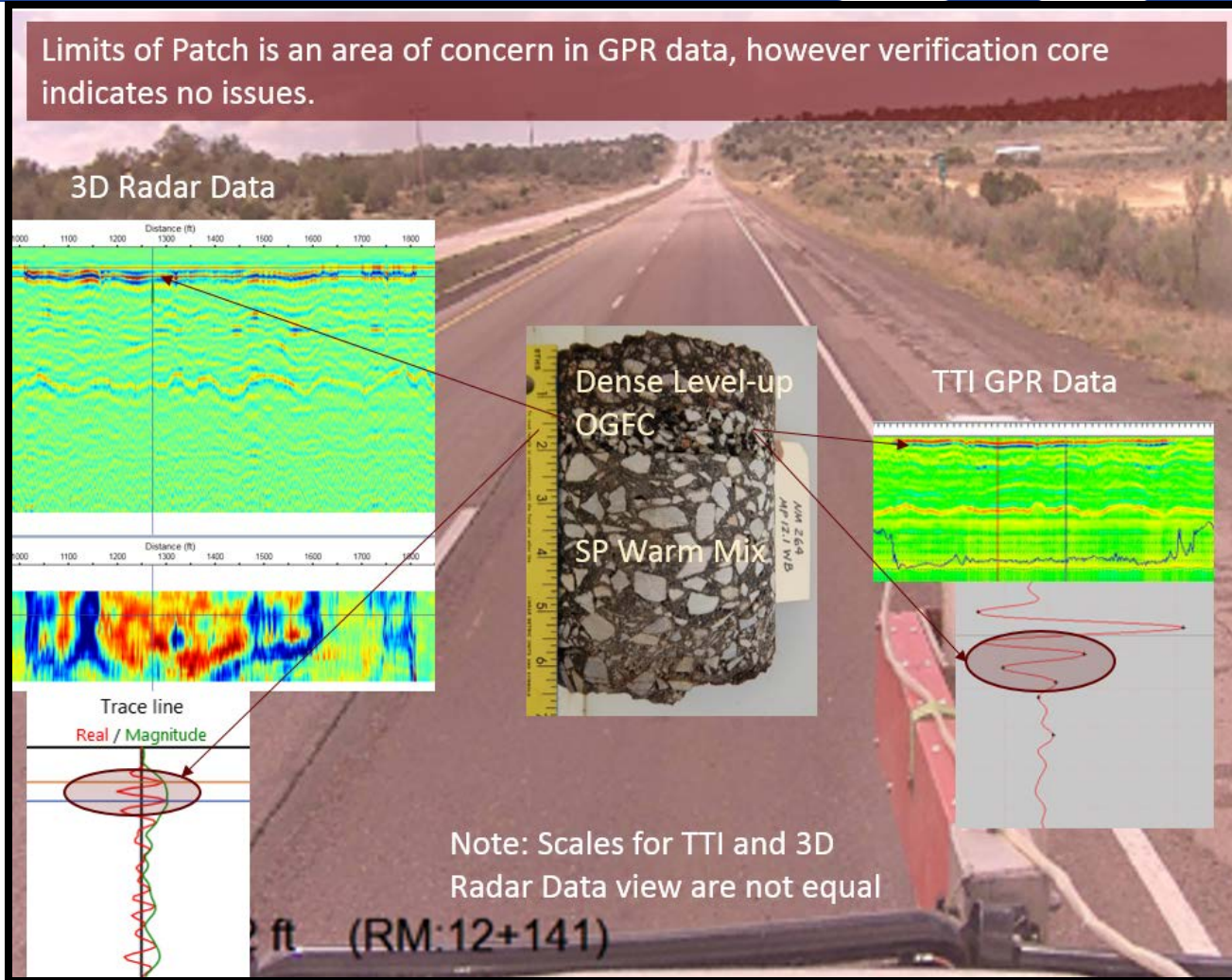




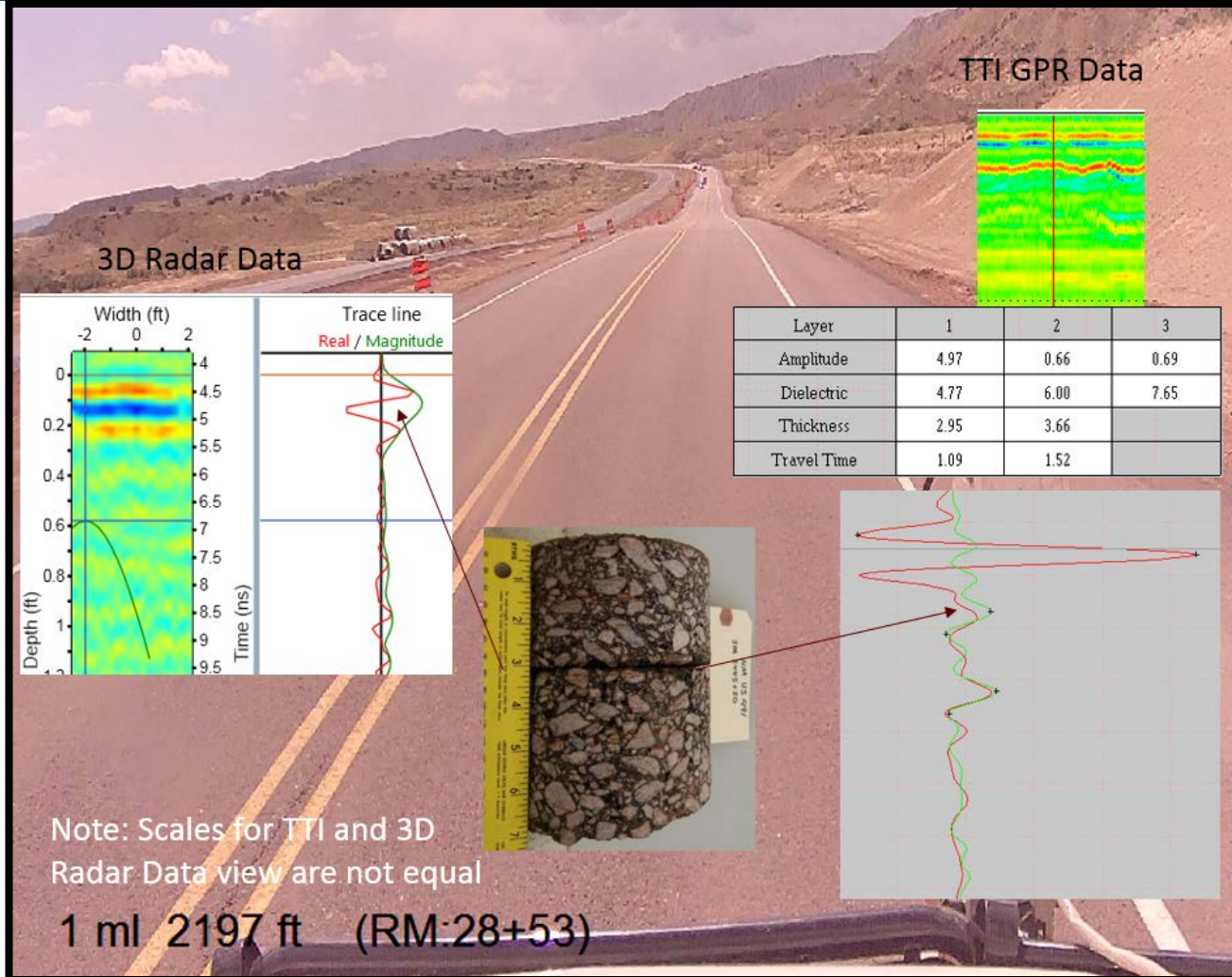
# Comparison – District 6



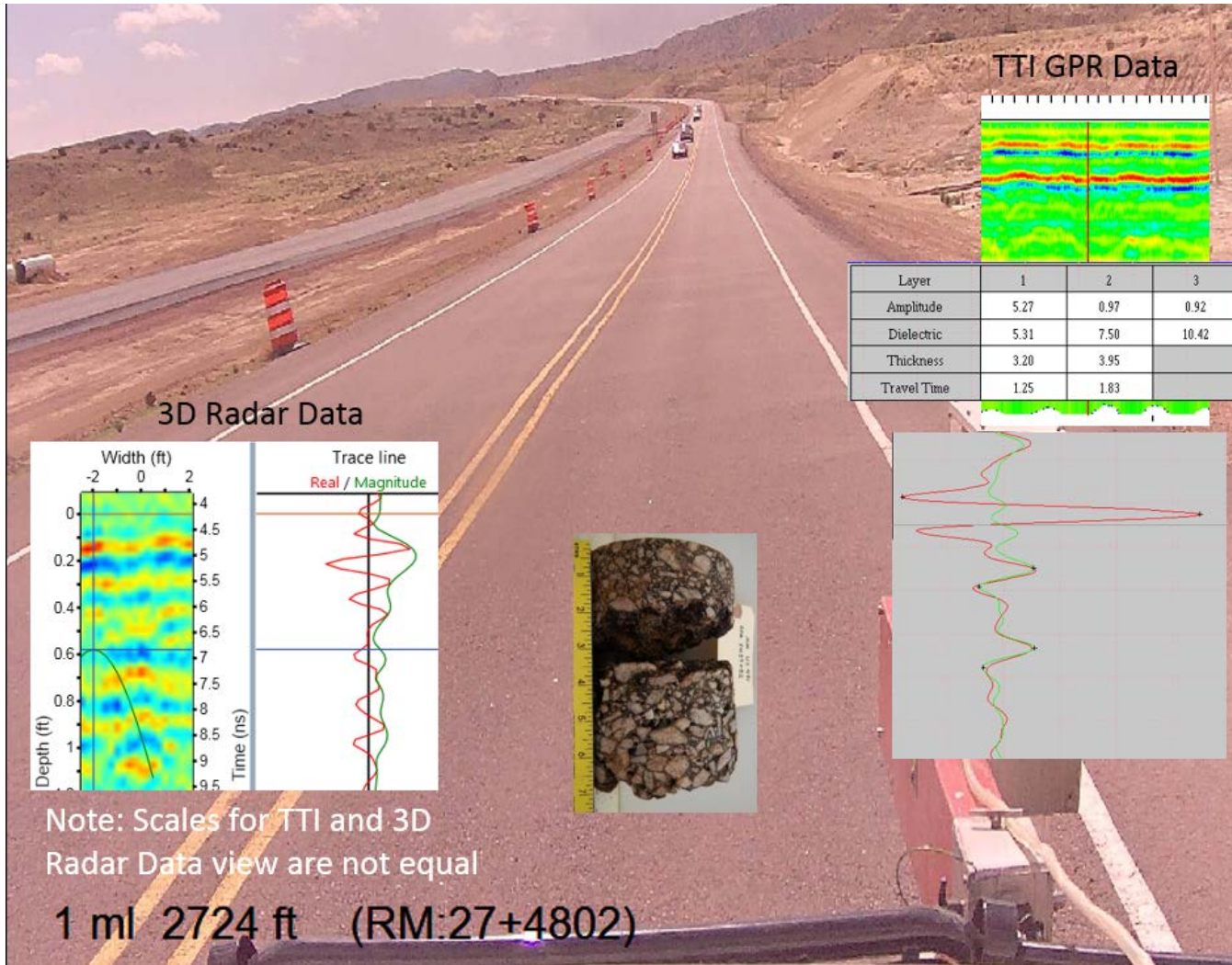
# Comparison - District 6



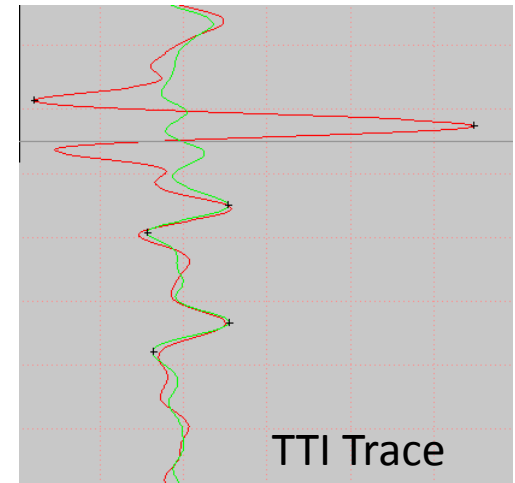
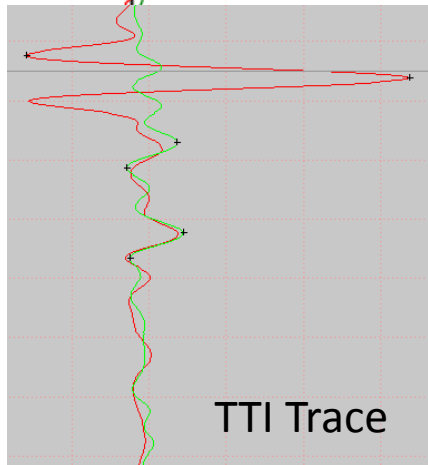
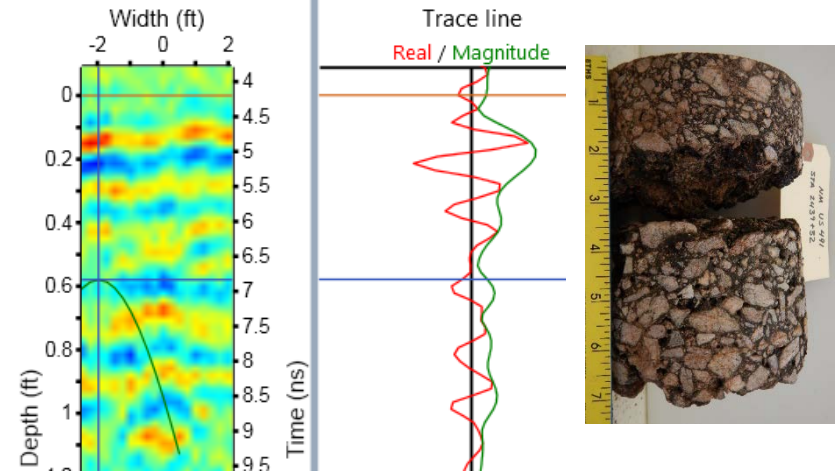
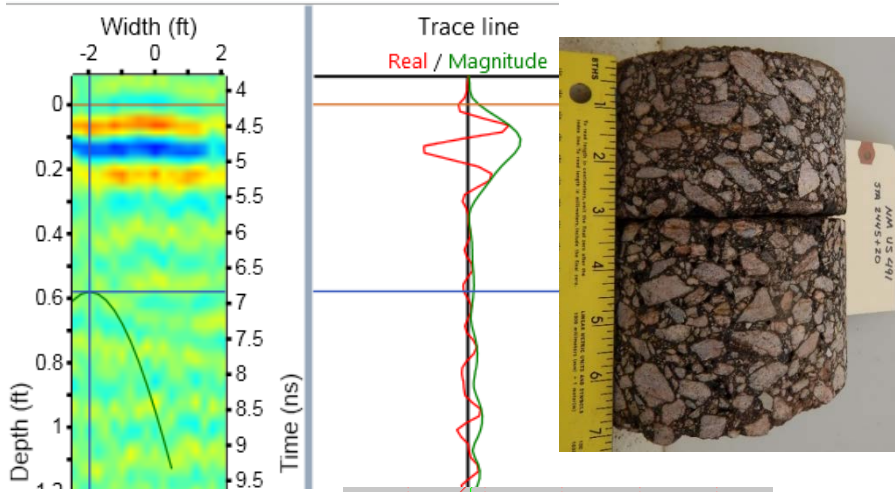
# Comparison – District 6



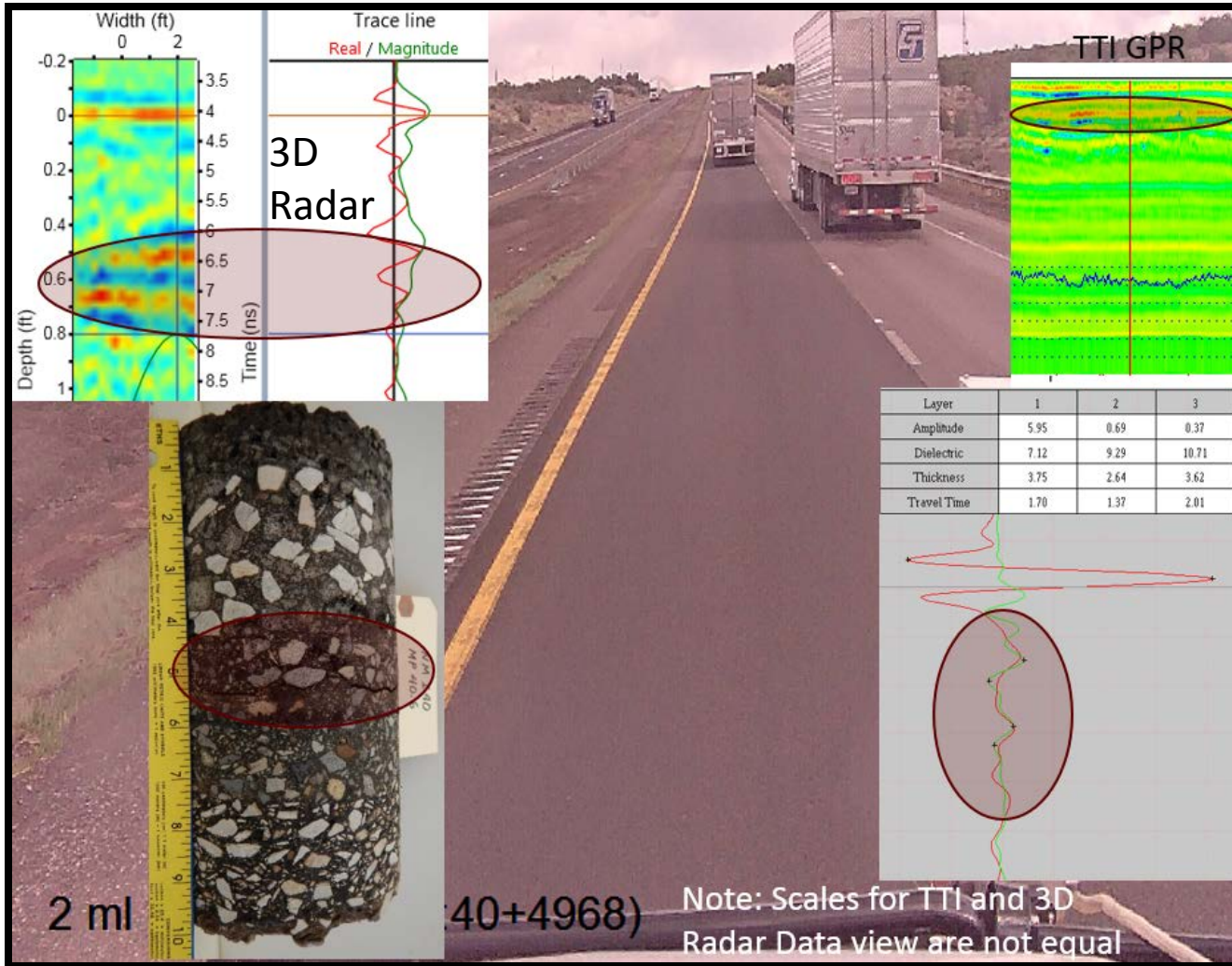
# Comparison – District 6



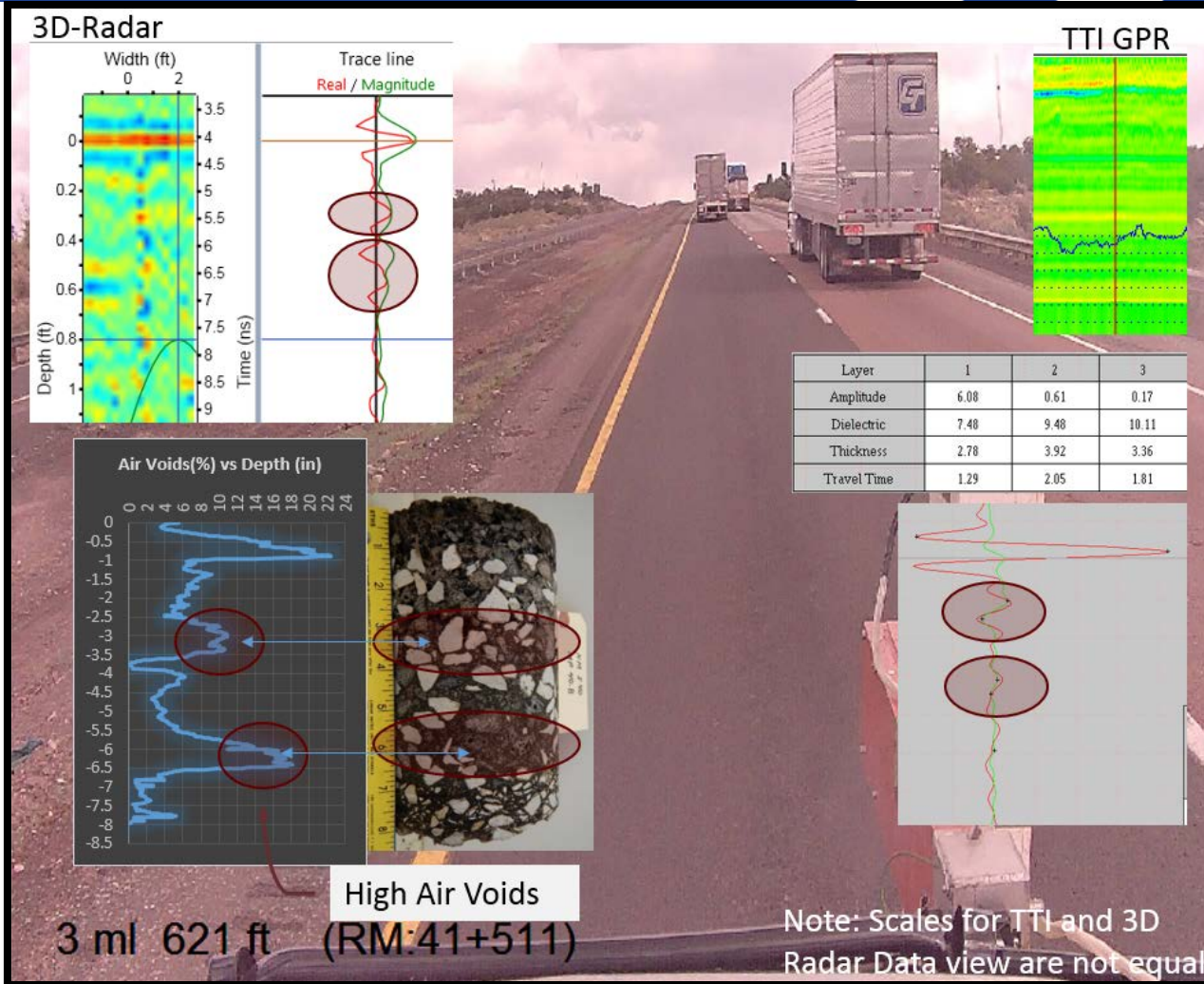
# Comparison – District 6



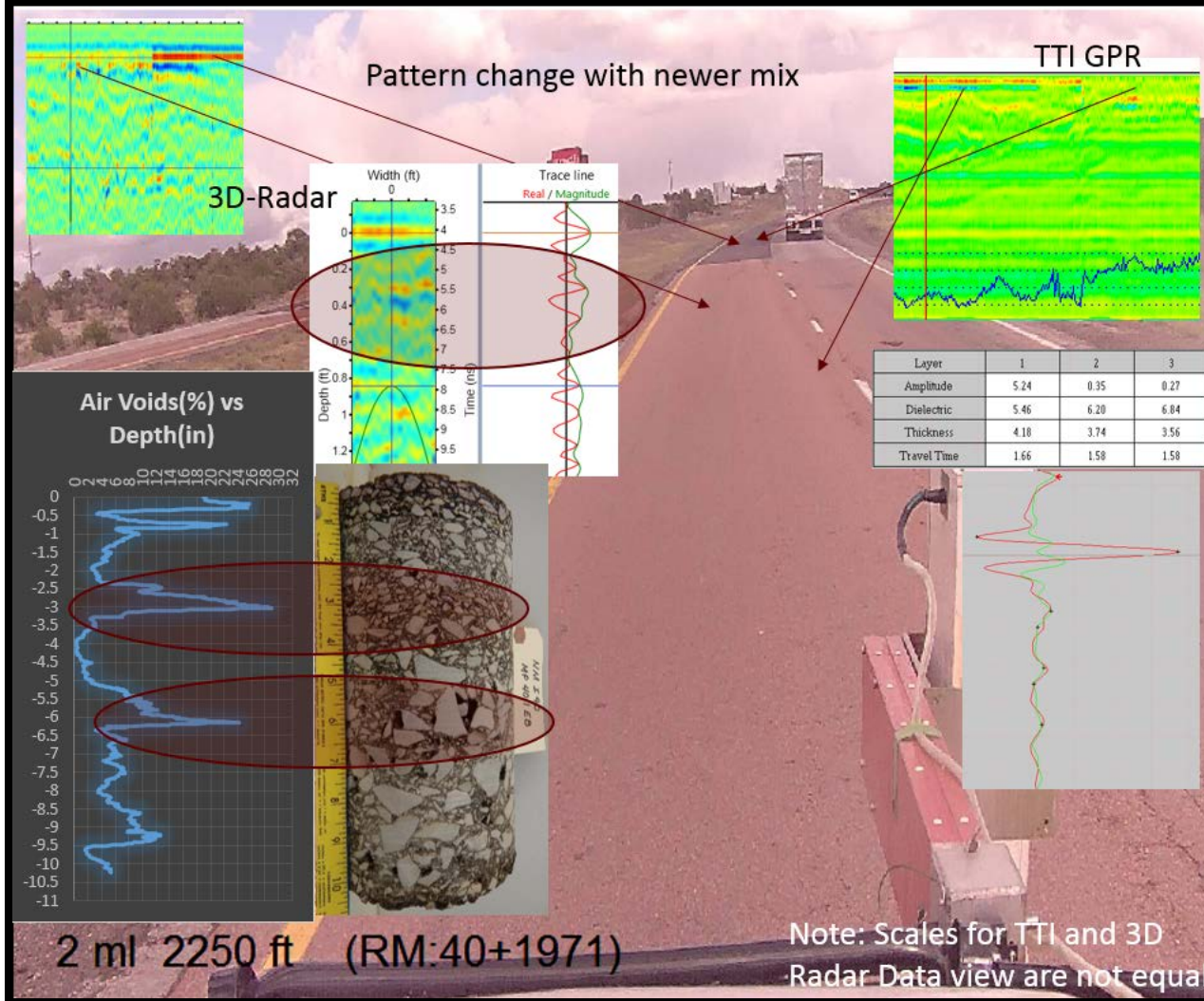
# Comparison – District 6



# Comparison – District 6

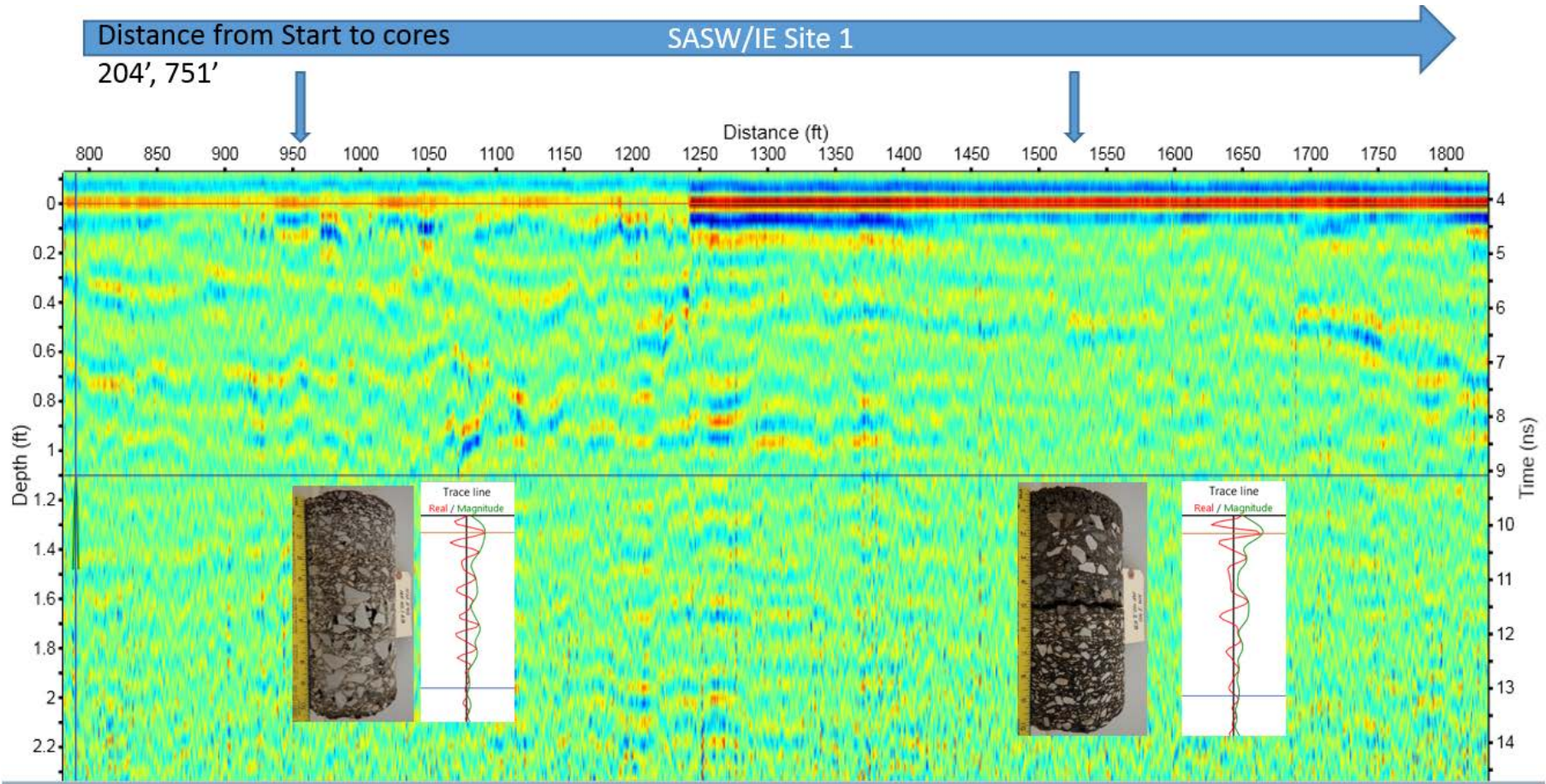


# Comparison – District 6

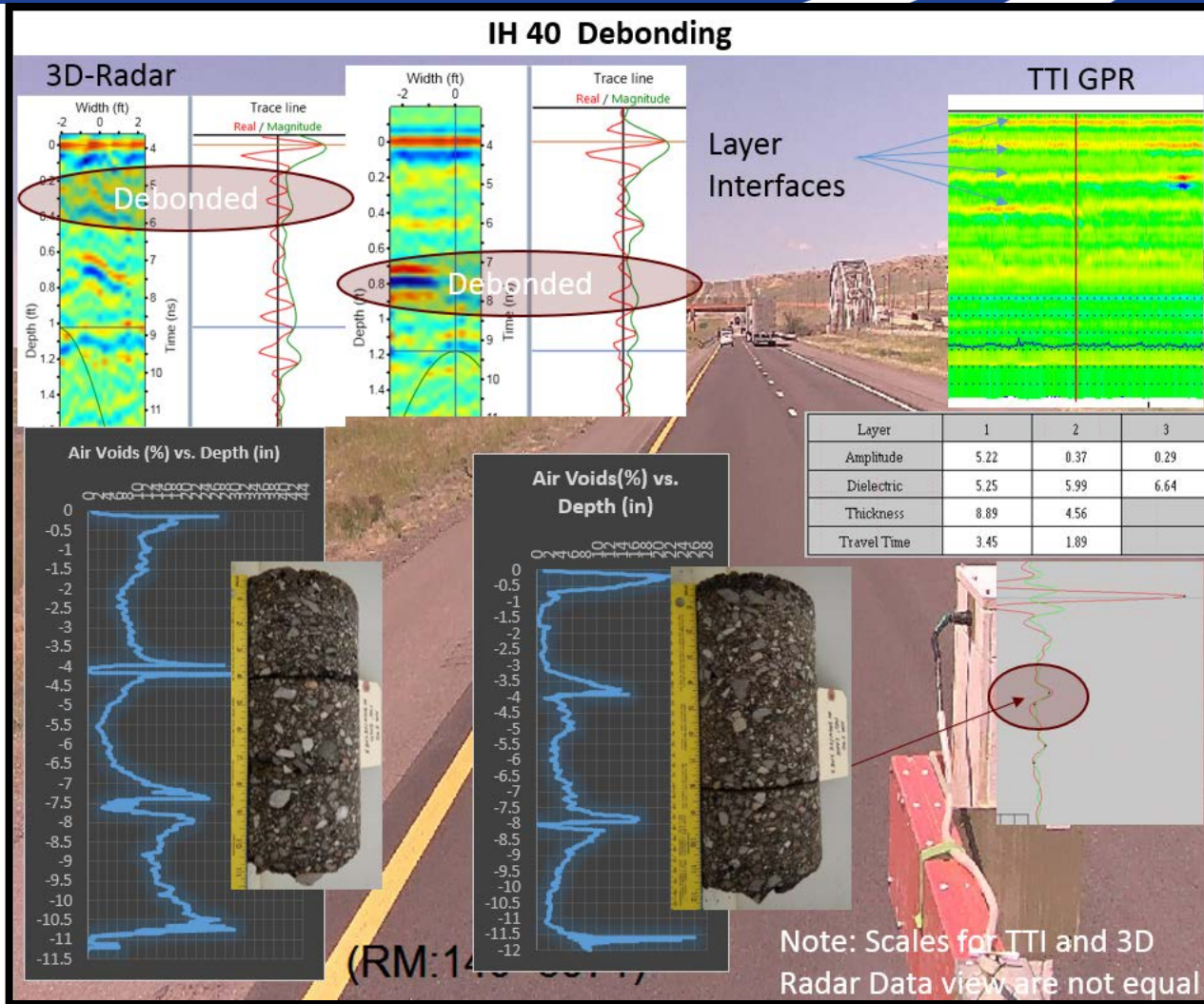




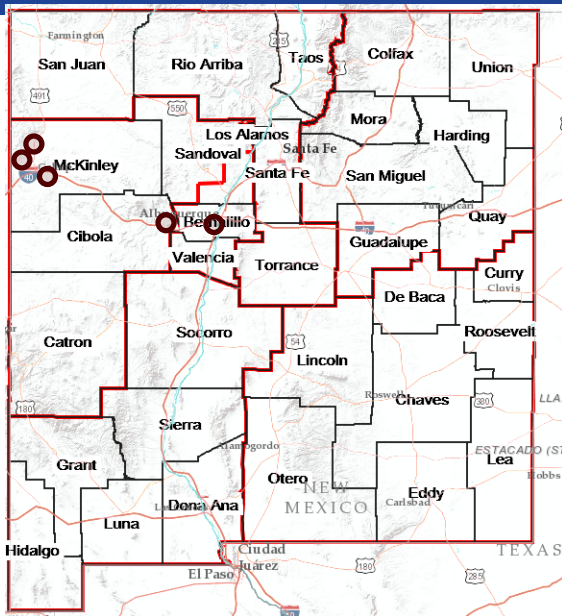
# Comparison – District 6



# Comparison – I40 District 3



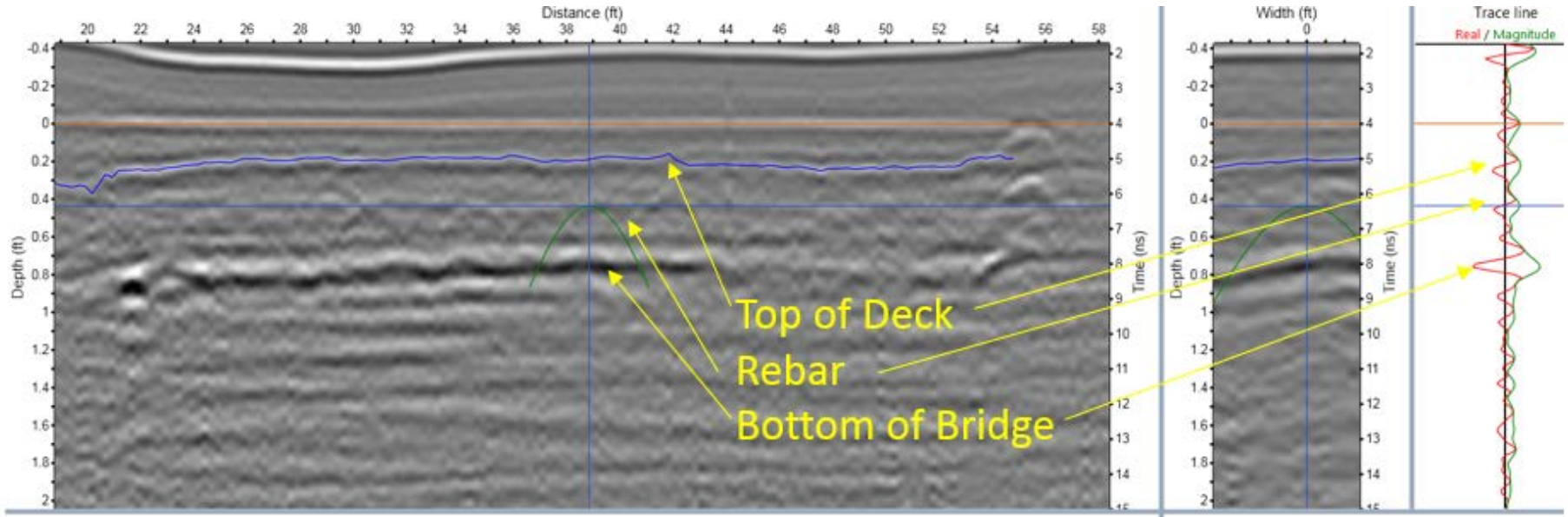
# GPR Test Locations – Other New Mexico



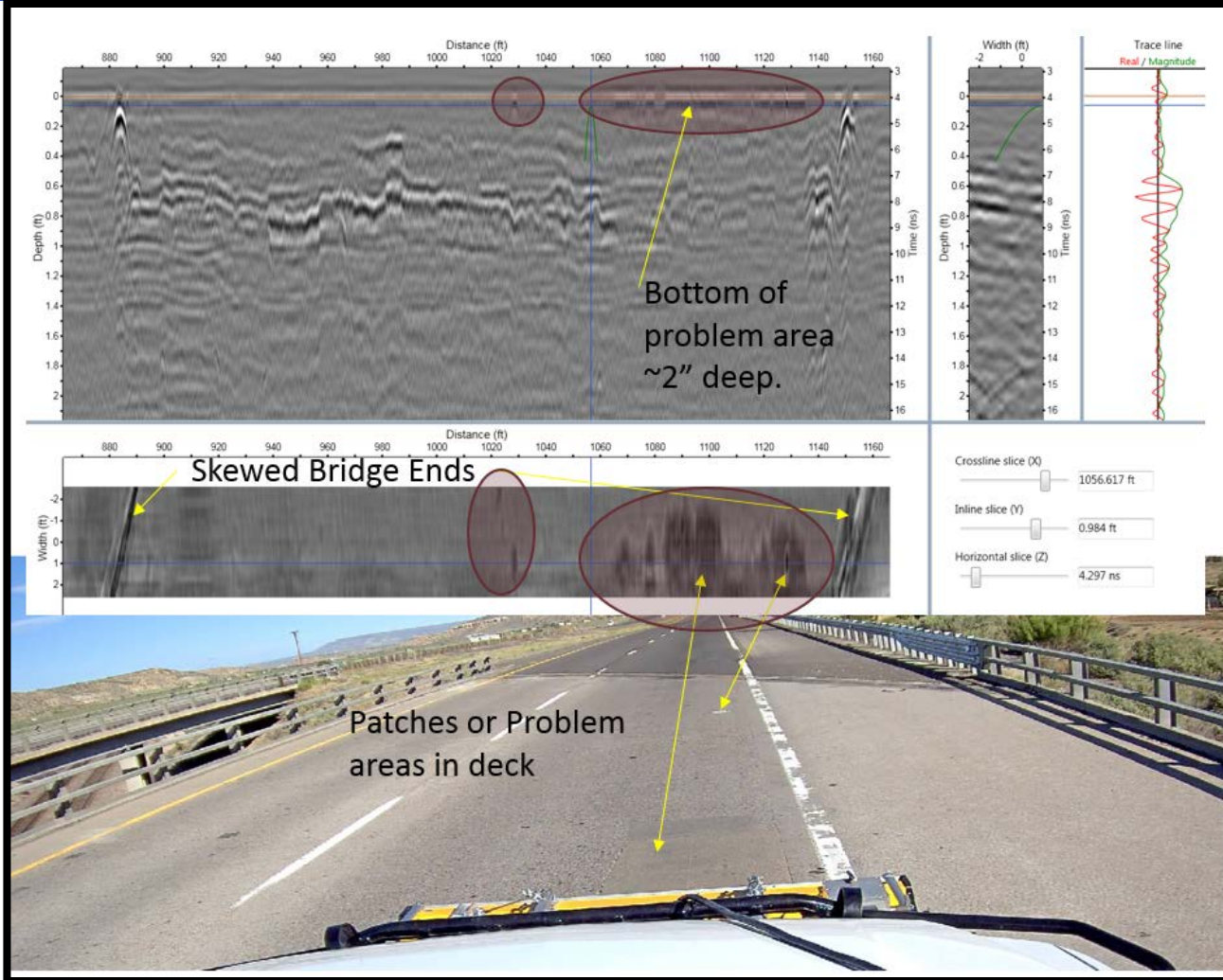
HWY	MP	MP	Comments
I-40	16	18	Conc – 4 lane
Jefferson Ave.	Metro Ave.	US491	Concrete
I-40	Ramps at Louisiana		Concrete

Bridge	Rdwy	Feature Intersected
9040	NM-309	BNSF Railroad
9013	61-Z000	Belen Highline Canal
6489	I-40 WBL	BNSF RAILROAD
8678	FR-4004	I-40 EBL/WBL @mp 39.9
6362	I-40 EBL	BNSF Railroad Spur
7157	NM-566	Rio Puerco/BNSF R/R
7158	NM-566	RIO PUERCO (NORTH FORK)

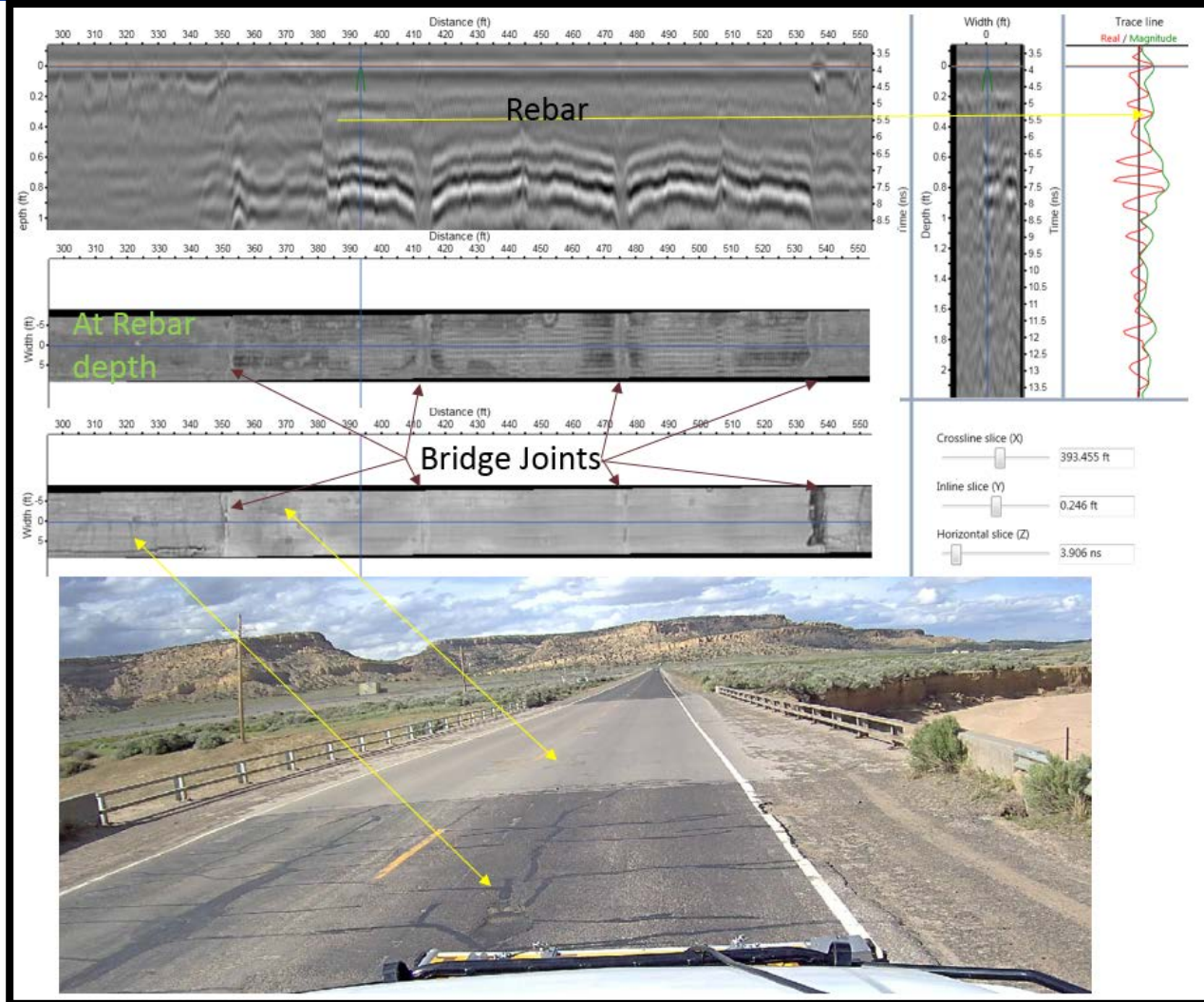
# Bridge with Overlay



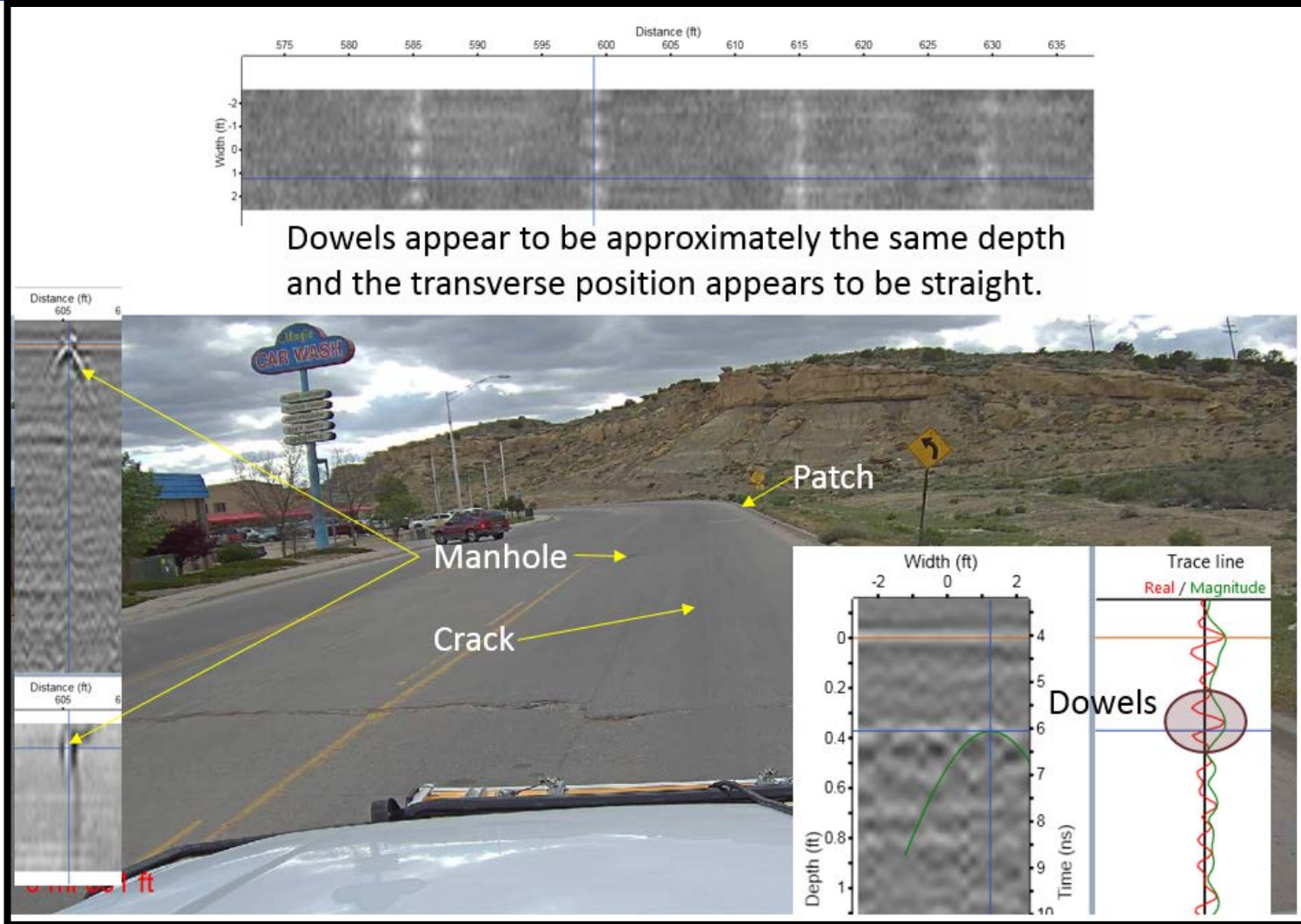
# Bridge with Patching



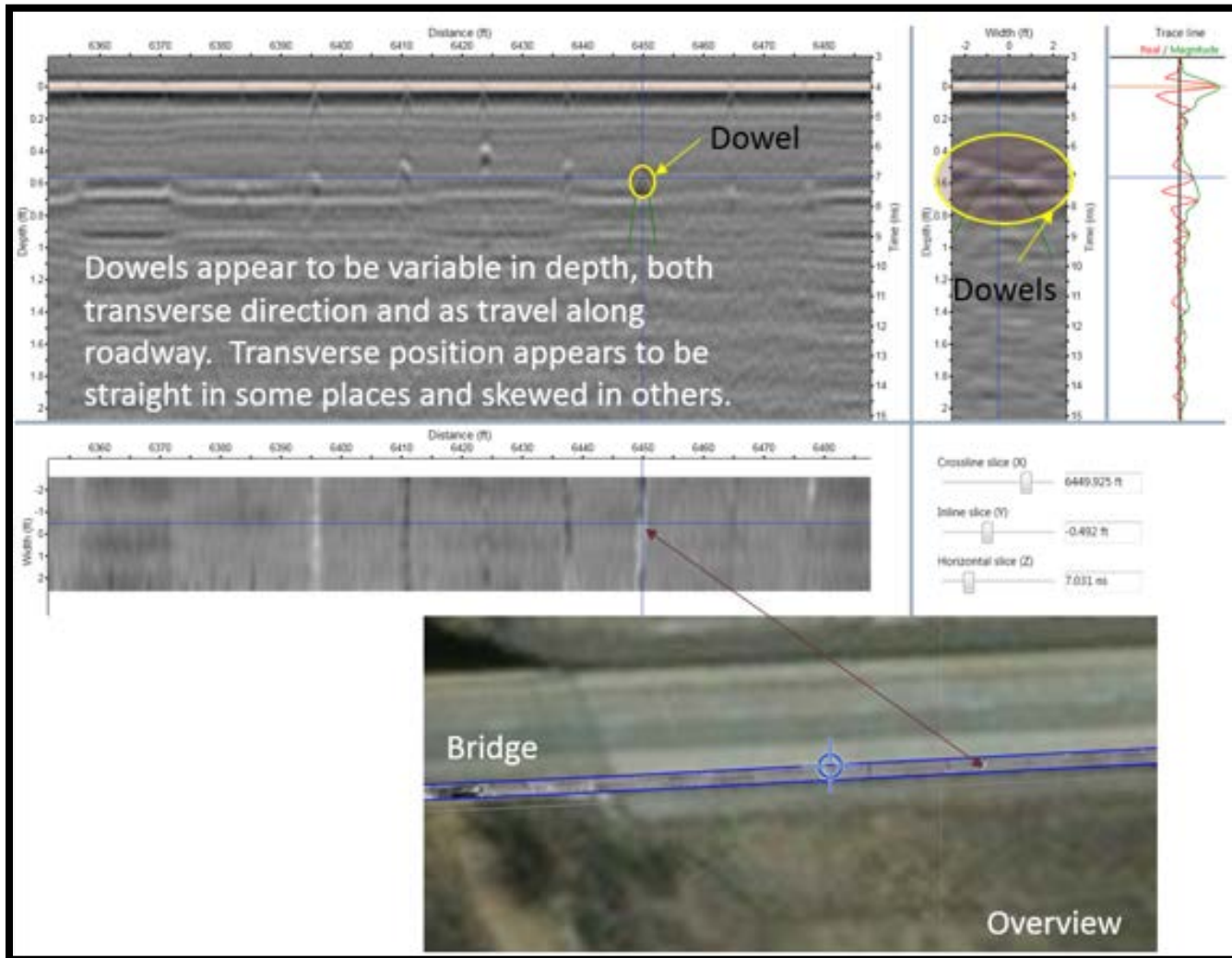
# Bridge



# Concrete Pavement

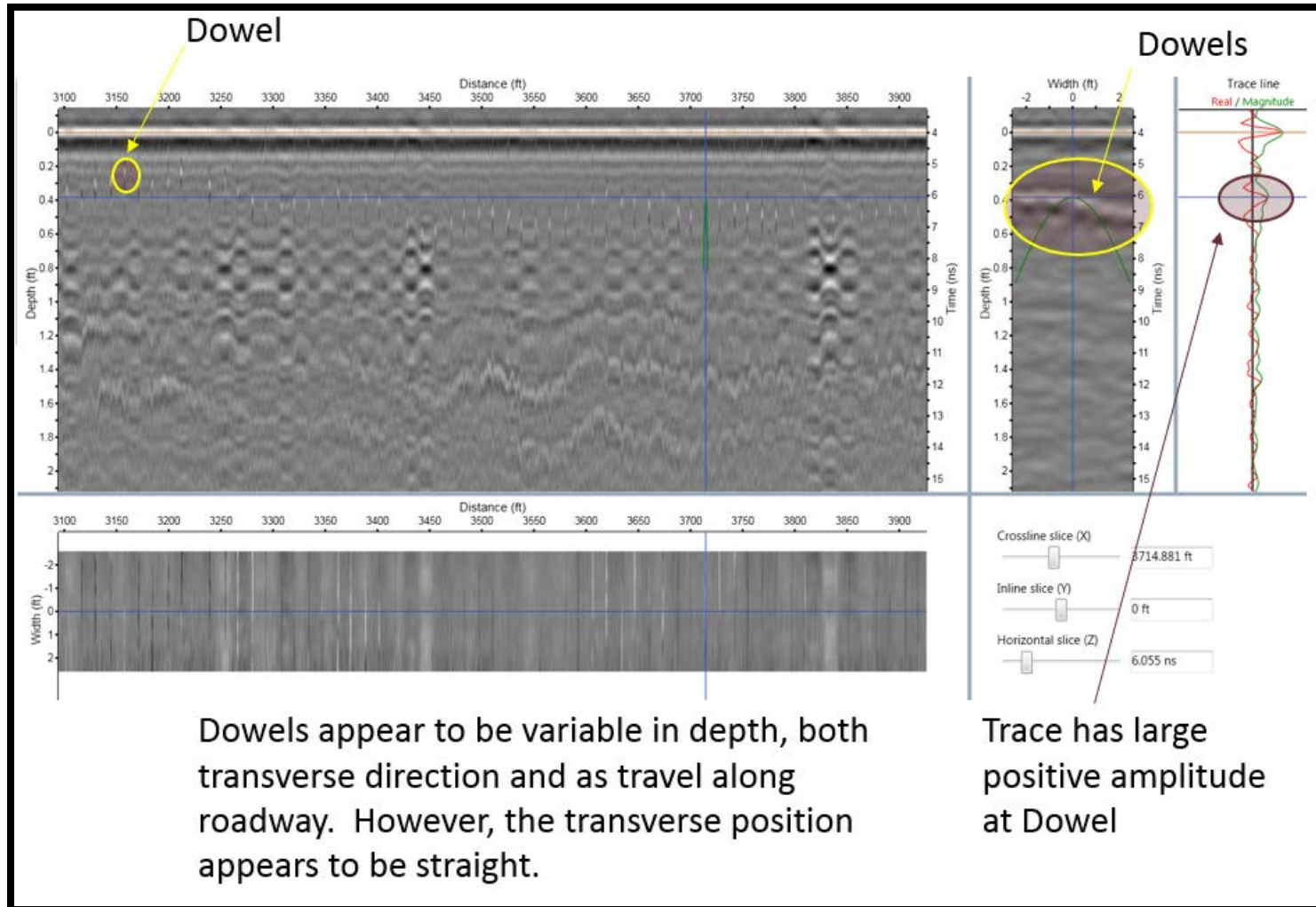


# Concrete Pavement





# Concrete Pavement



# Conclusions - Concrete Pavement

Some potential uses to help with the forensic evaluation of concrete pavement are:

- Thickness of pavement. In some cases, the bottom of the concrete can be found.
- Analyze orientation of Dowel Baskets or Dowels
  - Skewed
  - Level
  - Depth
    - both transversely and as travel down the roadway.

# Conclusions - Bridges

Some potential uses of this data to help with the forensic evaluation of bridge decks are:

- Depth of Bridge Deck overlay.
- Depth to Rebar.
- Find limits of surface irregularities
  - this was not able to be done on all bridges.
- Locations for Begin/End of Bridge and bottom of bridge deck.

# Conclusions - Flexible Pavement

- In general the patterns follow the patterns we expect based on past experience.
  - The false patterns encountered, help justify the need to take verification cores.
- It is very difficult to distinguish between severity of deterioration/delamination.
  - While the patterns are similar, severe stripping tends to have much larger amplitude.

# Recommendations

- Improve data storage efficiency for collection
- Examiner Software
  - Integrate video/images
  - Calculate dielectric and
  - Calculate layer thickness based on calculated dielectrics
- Continue to evaluate the 3D Radar System